



**PRECAST RETAINING WALL
SOLUTION**











ROAD MAP

- Insight
- Concept
- Advantages
- Components
- Details
- Theory
- Solutions



THE INSIGHT



“Why must we sacrifice space for stability?”

“What if bigger meant safer and faster, too?”





UNMISTAKABLE REDUCED LABOR
 RELIABLE ENGINEERED ORIGINAL

SOLID **SIMPLE** FAST SMART

BEAUTIFUL EFFECTIVE DIMENSION

ROBUST 10/10

BIG SAFE INNOVATIVE LIGHT

PROVEN AESTHETIC

PRODUCTIVE GRAVITY

DURABLE

STABLE

VERSITILE



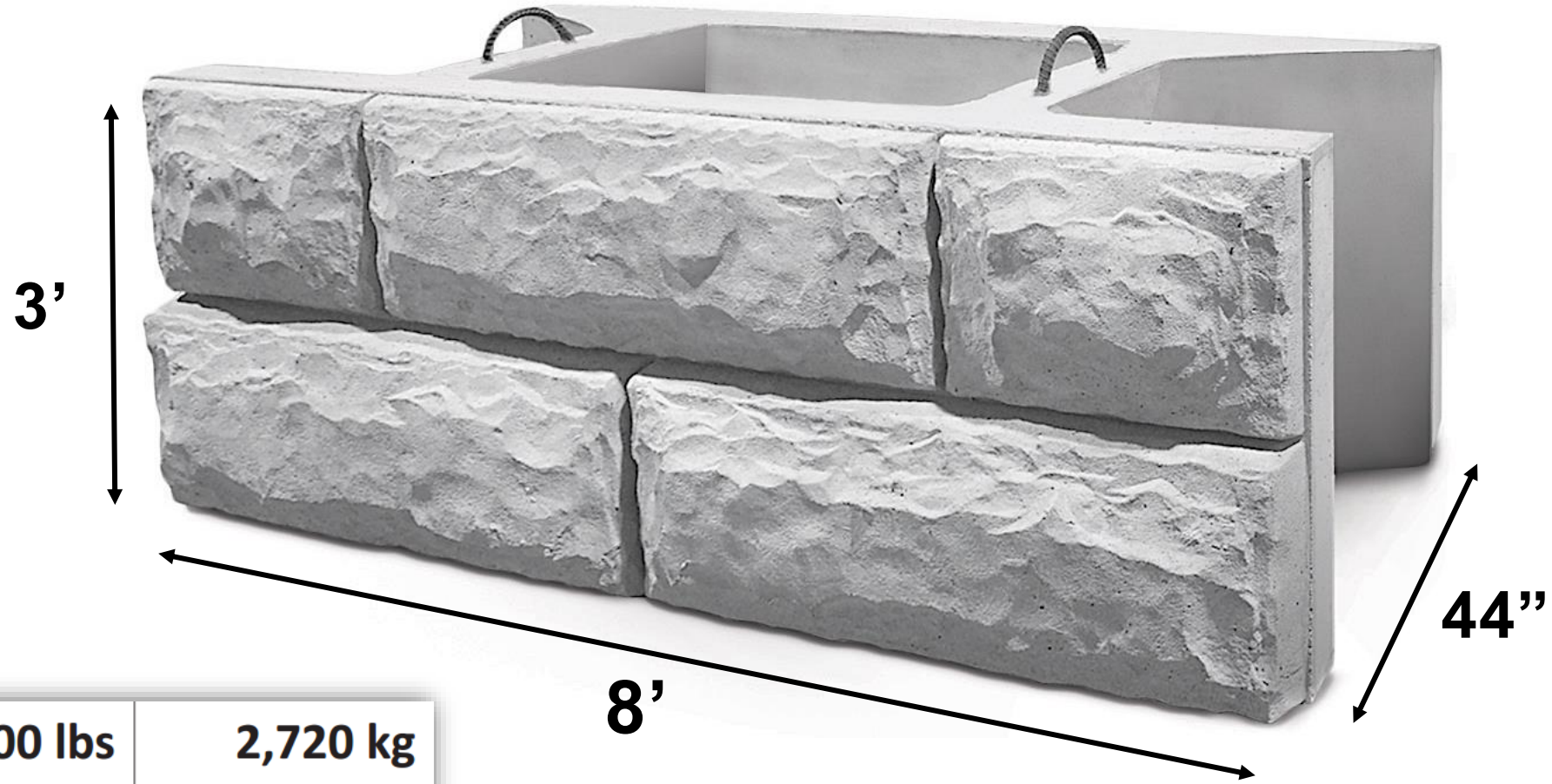
THE CONCEPT



Create a fully and intelligently engineered retaining wall system.



24 SF BLOCK



| | | |
|------------------|-------------|-------------------------|
| Block Wt. | 6,000 lbs | 2,720 kg |
| Form Wt. | 4,600 lbs | 2,090 kg |
| Concrete Volume | 1.50 CY | 1.15 m ³ |
| Aggregate Infill | 1.60 CY | 1.22 m ³ |
| (per face area) | 0.1 tons/sf | 1,000 kg/m ² |





NOT JUST BIGGER.

BETTER.

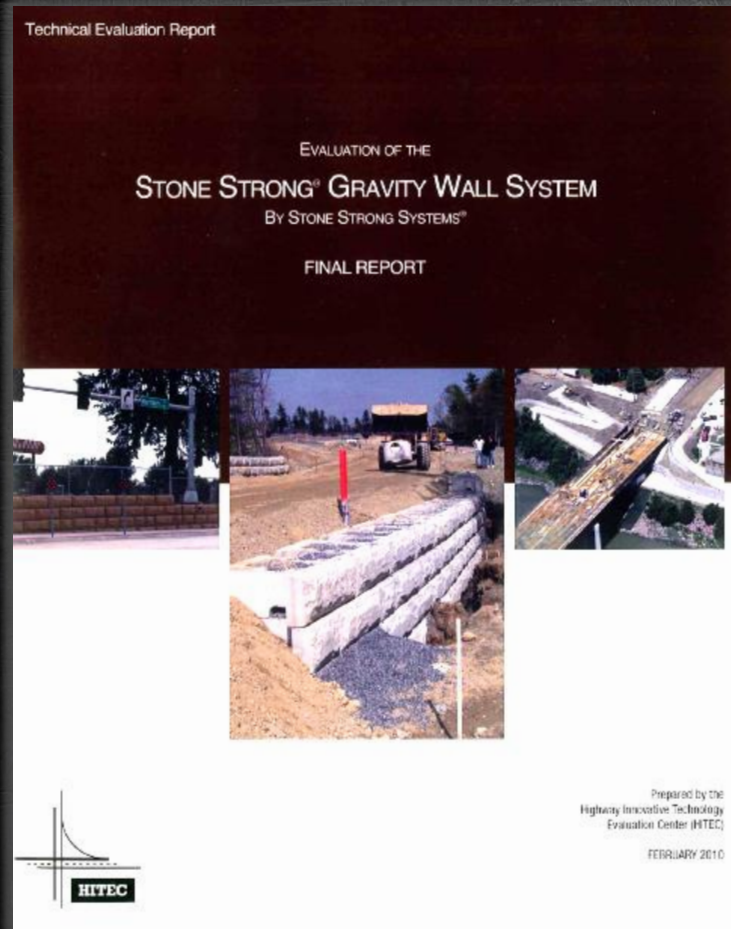


**DON'T JUST TAKE
OUR WORD FOR IT...**



HITEC Evaluation

Highway Innovative Technology Evaluation Center



Key Findings

- **Viable alternative to traditional cast in place and MSE retaining wall systems**
- **Validation of methodologies and underlying engineering basis and concepts**
- **Evidence of conformance with AASHTO methodologies**



ASTM C1776

This international standard was developed in accordance with internationally recognized principles on standardization established in the Decision on Principles for the Development of International Standards, Guides and Recommendations issued by the World Trade Organization Technical Barriers to Trade (TBT) Committee.



Designation: C1776/C1776M – 17

Standard Specification for Wet-Cast Precast Modular Retaining Wall Units¹

This standard is issued under the fixed designation C1776/C1776M; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon (ϵ) indicates an editorial change since the last revision or reapproval.

1. Scope

1.1 This specification covers wet-cast precast modular retaining wall units cast from first-purpose concrete with or without the inclusion of steel reinforcement. The precast units covered by this specification are machine-placed units intended for use in the construction of dry stacked modular retaining wall systems.

[A884/A884M](#) Specification for Epoxy-Coated Steel Wire and Welded Wire Reinforcement

[A934/A934M](#) Specification for Epoxy-Coated Prefabricated Steel Reinforcing Bars

[A1055/A1055M](#) Specification for Zinc and Epoxy Dual Coated Steel Reinforcing Bars

[A1060/A1060M](#) Specification for Zinc-Coated (Galvanized) Steel Welded Wire Reinforcement, Plain and Deformed





STONE
STRONG
SYSTEMS®



THE ADVANTAGES



BIGGER

4X LARGER than the competition.

- Massive 24-square-foot block
- Structural strength without need for mechanical tieback
- Reduced labor costs
- Faster, easier installation process



LIGHTER

Install **1,200** square feet
of block per day with a three-man crew.

- Reduced transportation costs
- Innovative hollow design
- Faster, easier and less costly installation process
- Less weight per square foot



SMARTER

Full structural and geotechnical engineering

- Voids provide integrated drainage system
- Aggregate infill ensures interlock between courses
- Lifting loops align precisely
- Recesses deliver automatic setback
- Tapered edges contour to any landscape
- Dense, less permeable air- entrained 4,000-psi concrete-minimum



AESTHETICALLY **ATTRACTIVE**

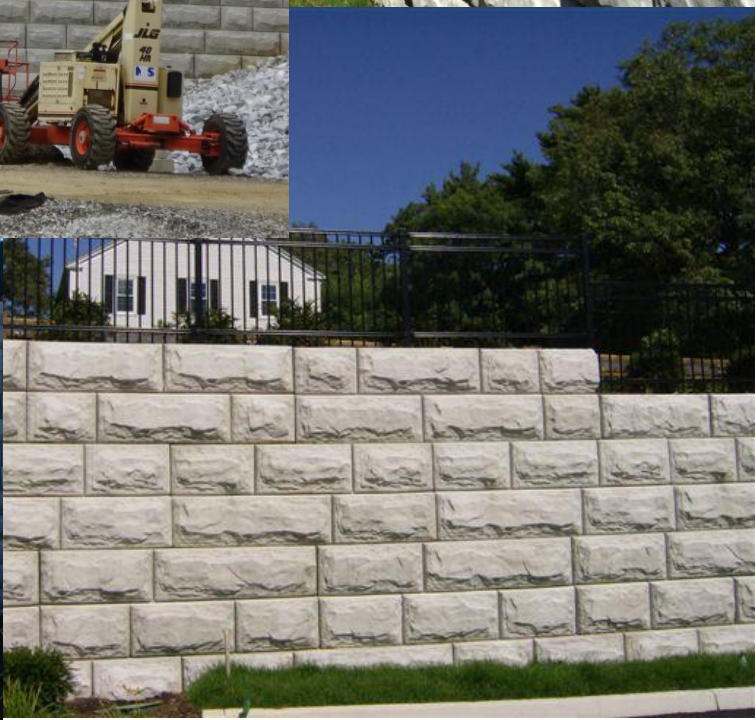
BIG AND BEAUTIFUL

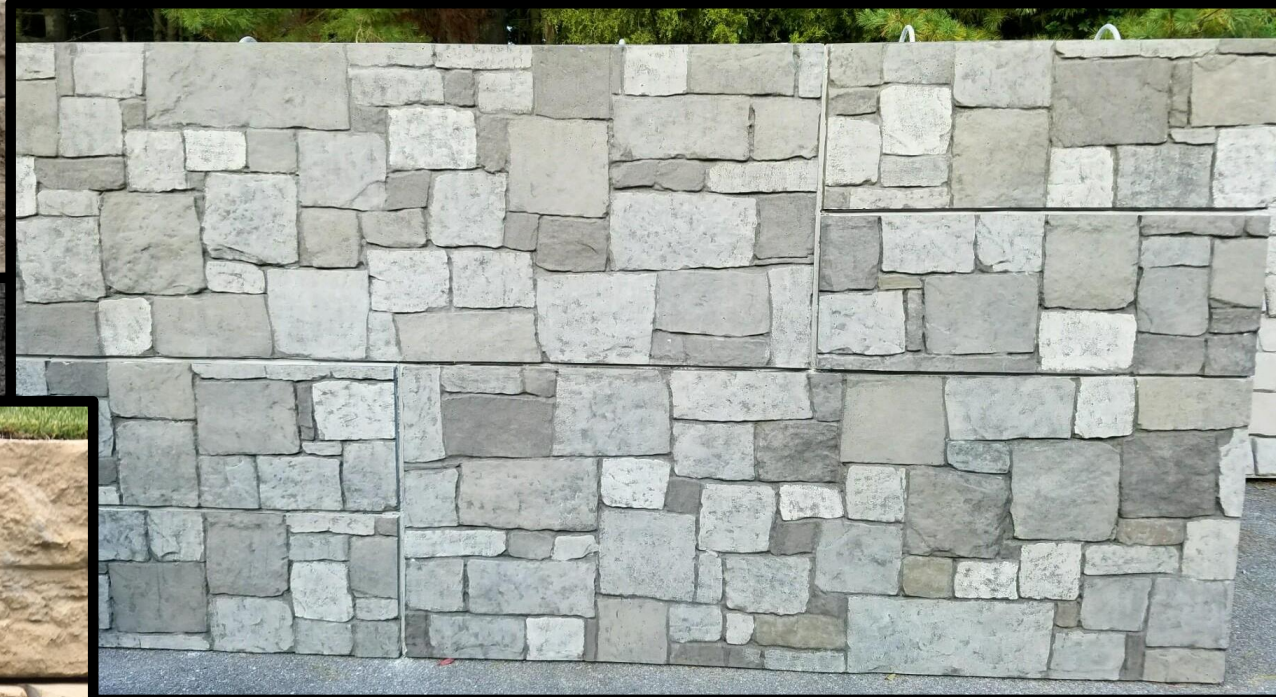
Customizable patterns

Handcrafted by real artisans

Fits seamlessly into any landscape









Endless Possibilities



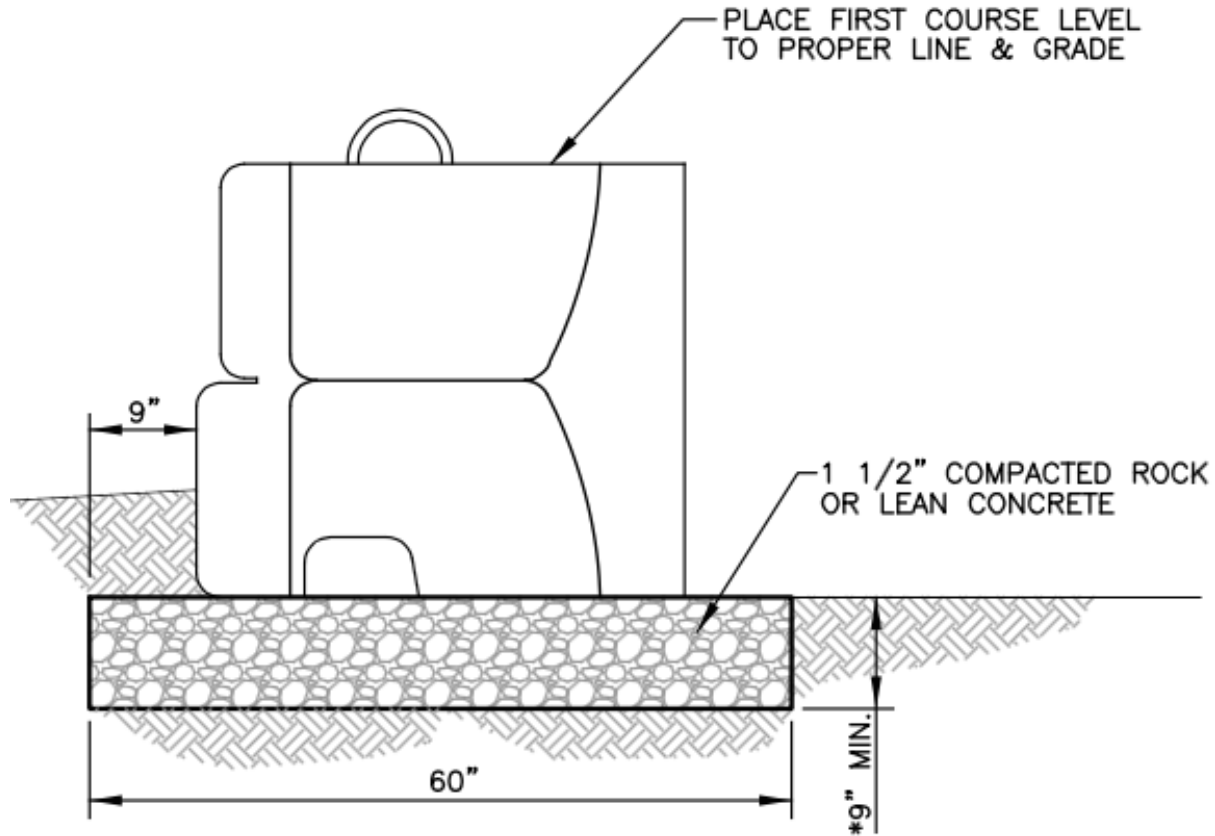
THE COMPONENTS



24 SF Block

Face: 8' x 3'; Width 44"

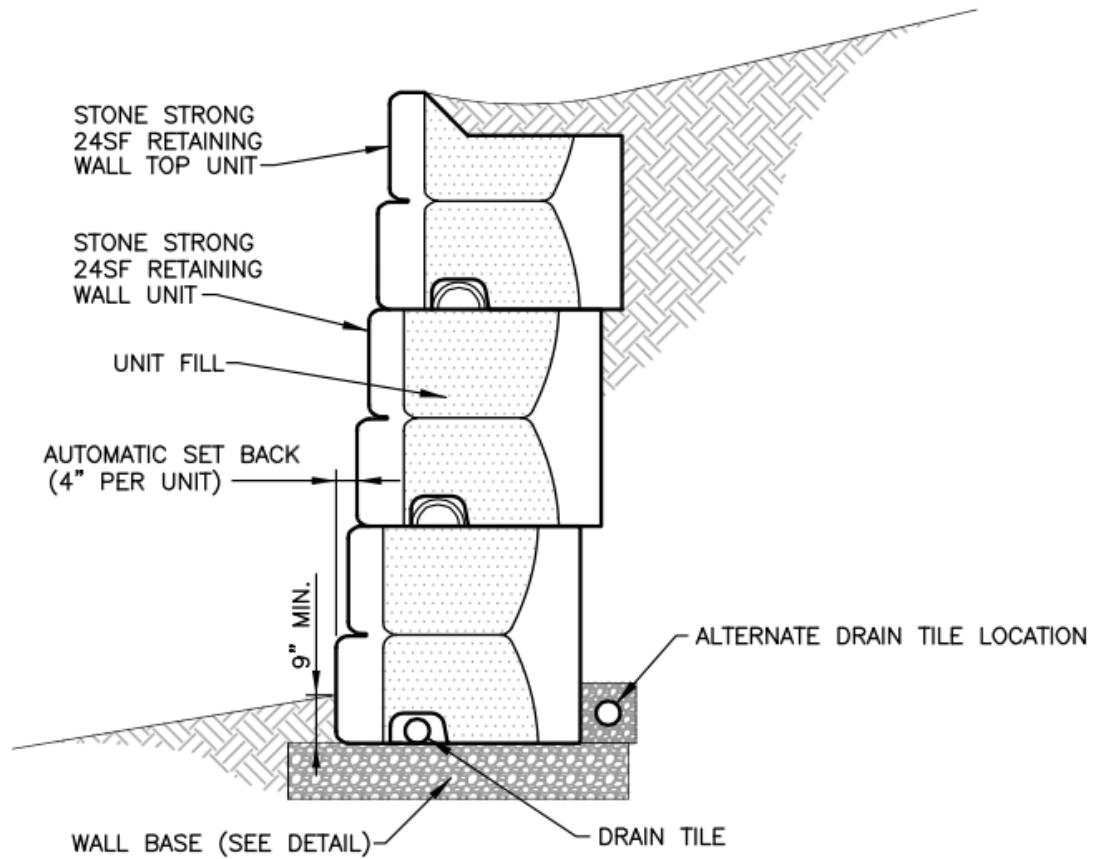
Weight: 6,000 lbs.



24 SF Top Block

Face: 8' x 3'; Width 44"

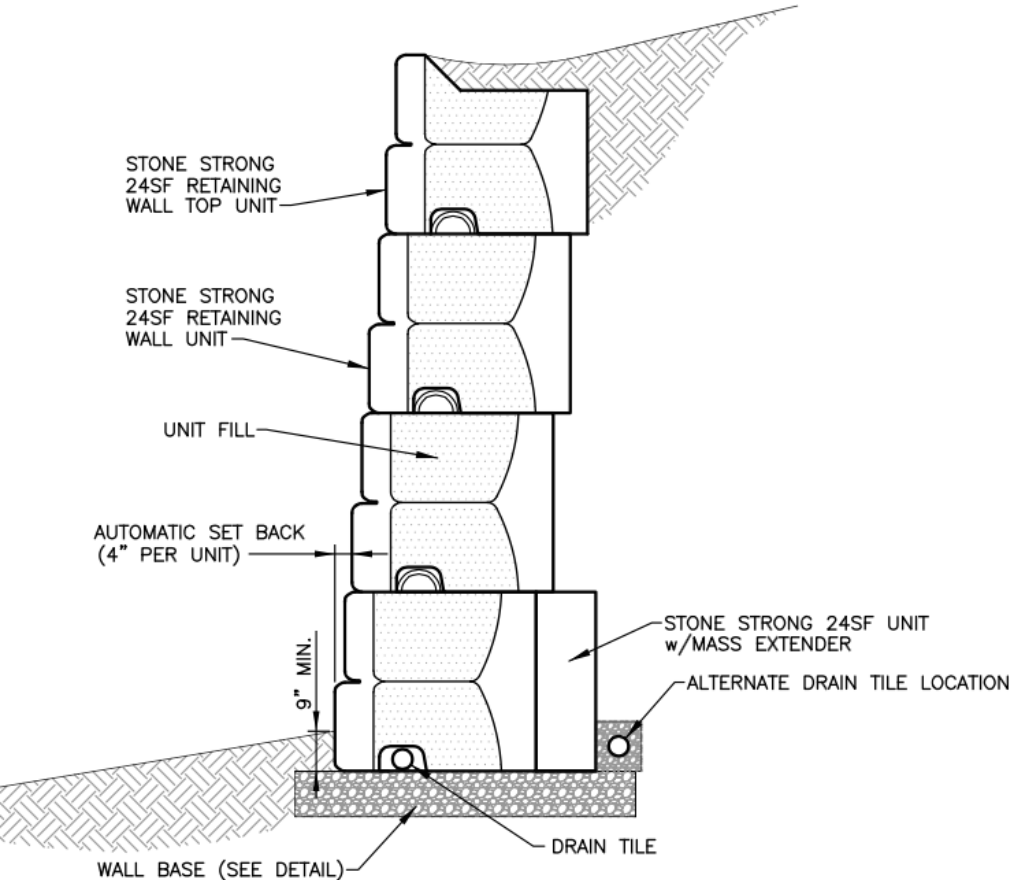
Weight: 5,400 lbs.



24 SF Mass Extender Block

Face: 8' x 3'; Width 56"

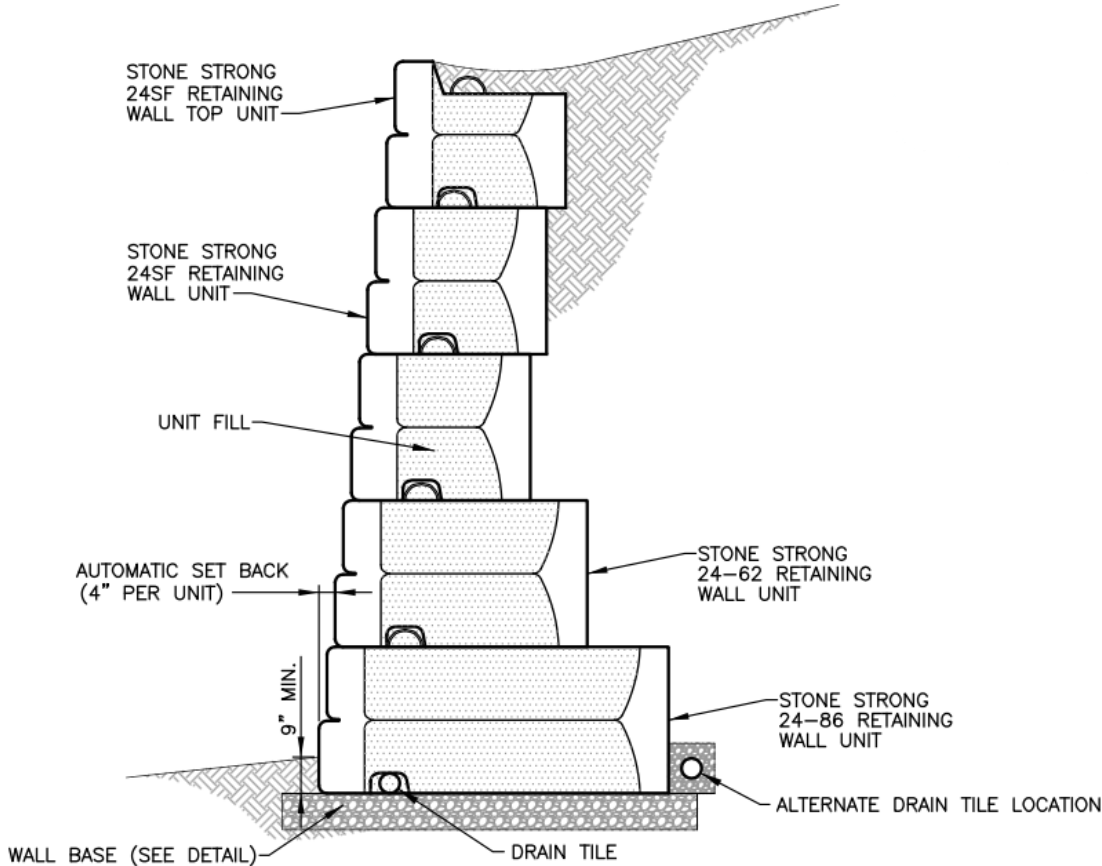
Weight: 10,000 lbs.



24-62 Block

Face: 8' x 3'; Width 62"

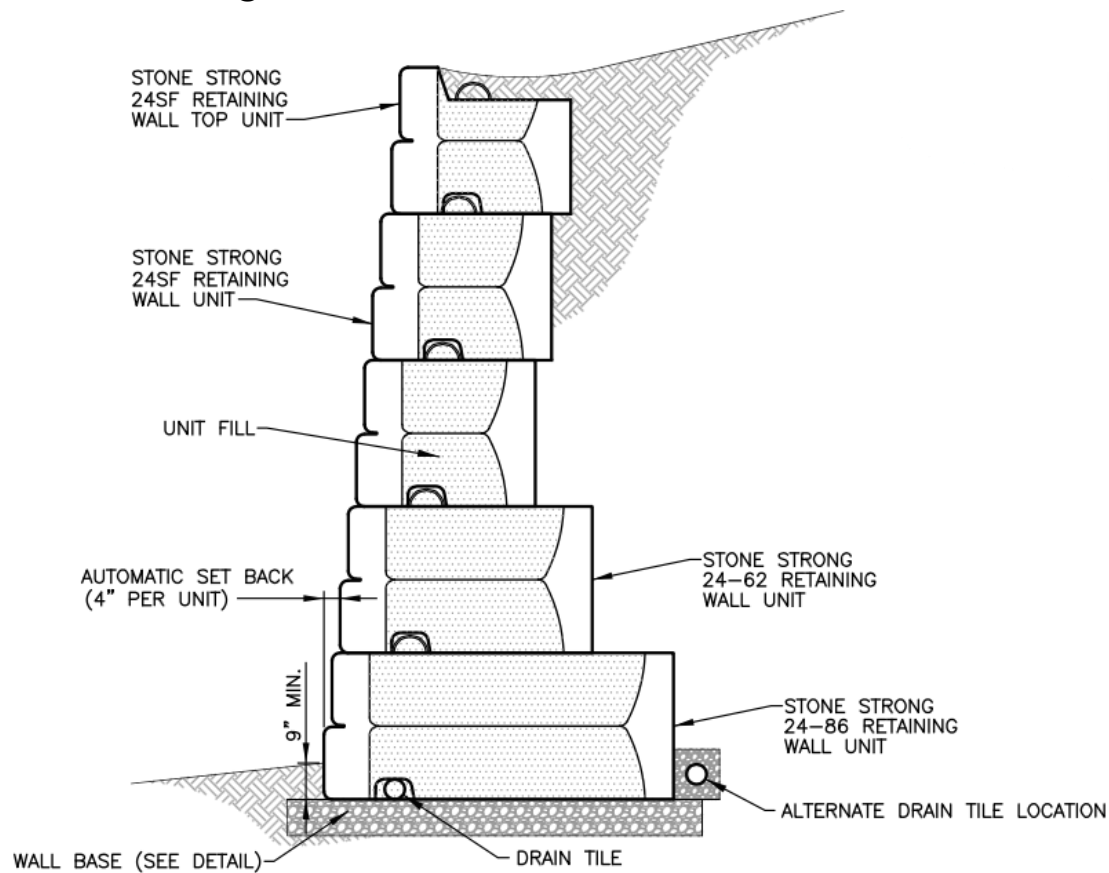
Weight: 6,600 lbs.



24-86 Block

Face: 8' x 3'; Width 86"

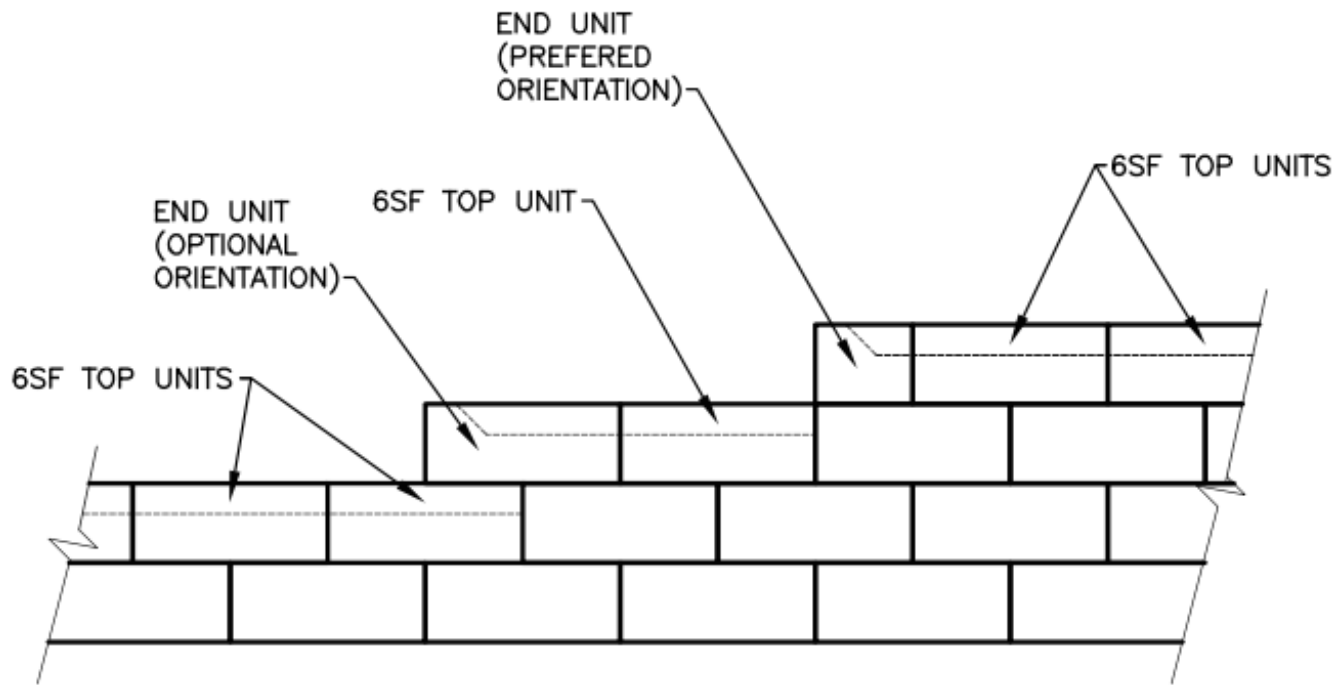
Weight: 7,400 lbs.



6 SF Block

Face: 4' x 18"; Width 44"

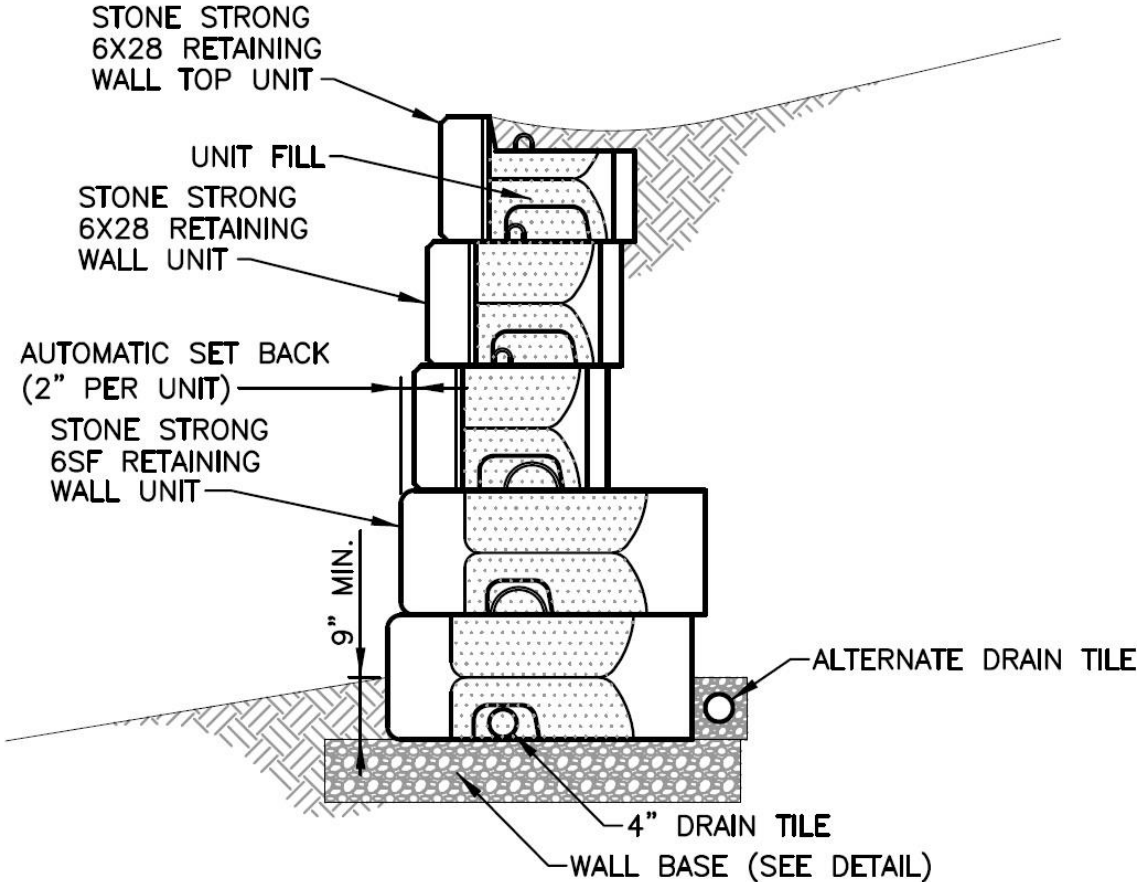
Weight: 1,600 lbs.



6-28 Block

Face: 4' x 18"; Width 28"

Weight: 950 lbs.



Accessories



3 SF Block



45° Block



90° Block



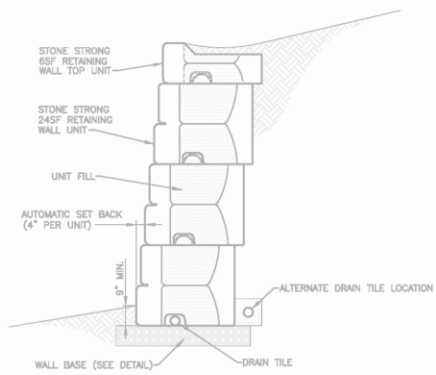
End/Corner Block



Dual Face Block

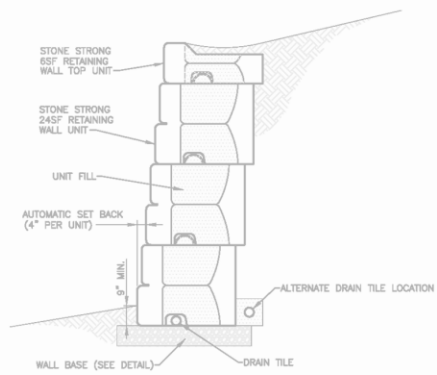


- NOTES:**
1. DRAIN TILE MAY BE ELIMINATED AT THE DISCRETION OF THE SITE ENGINEER.
 2. DAYLIGHT DRAIN TILE AT LOW POINTS THROUGH WALL FACE OR AT ENDS; MAXIMUM SPACING 100 FEET OR PER SITE CONDITIONS.



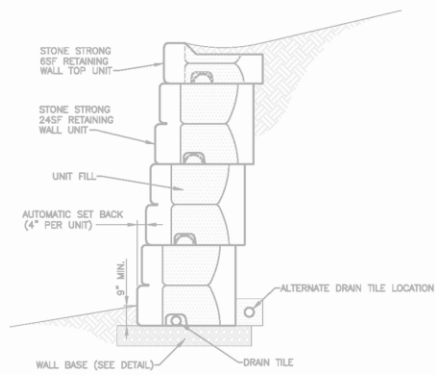
GRAVITY WALL CROSS SECTION
NOT TO SCALE

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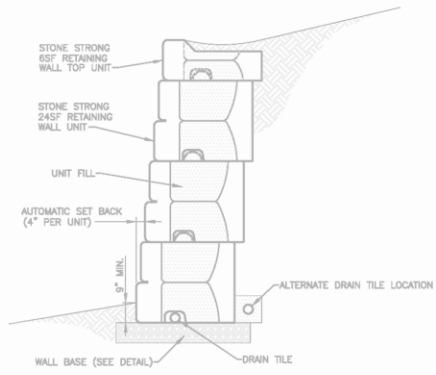
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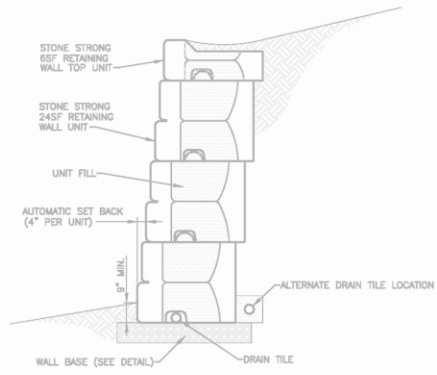
THE DETAILS

- NOTES:**
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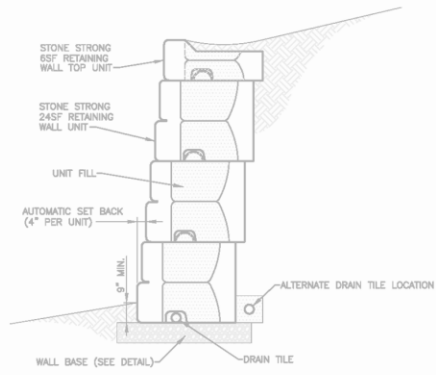
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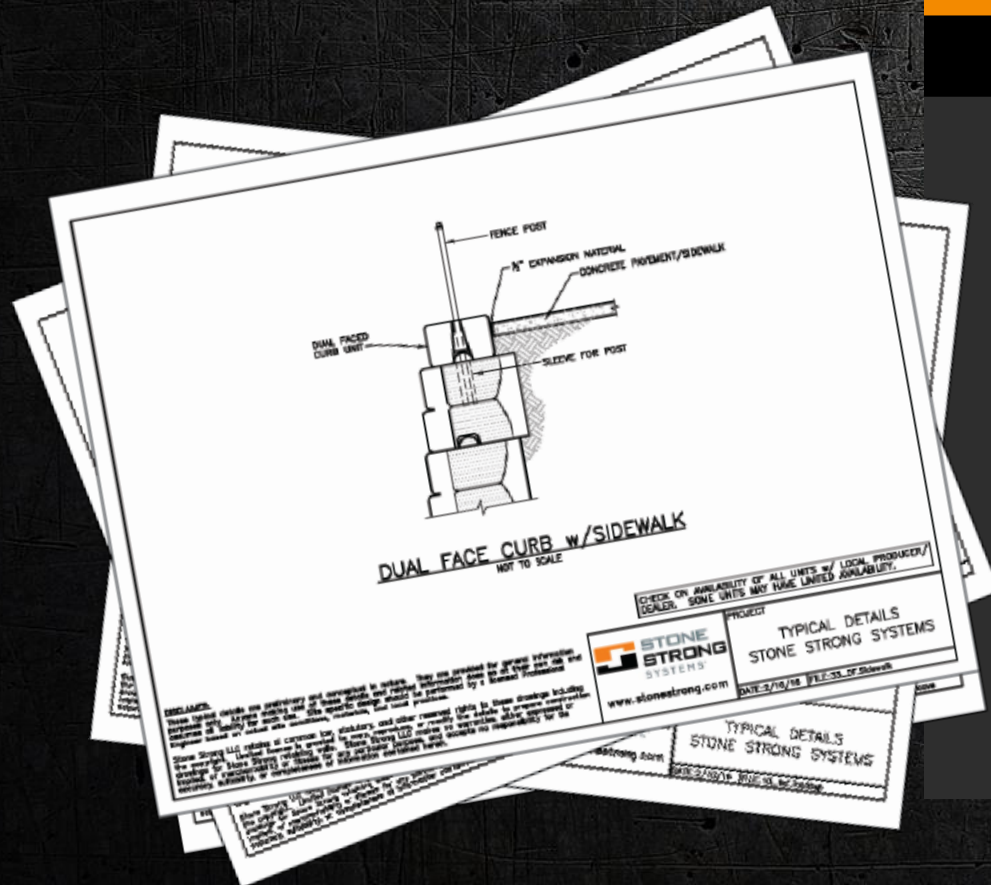
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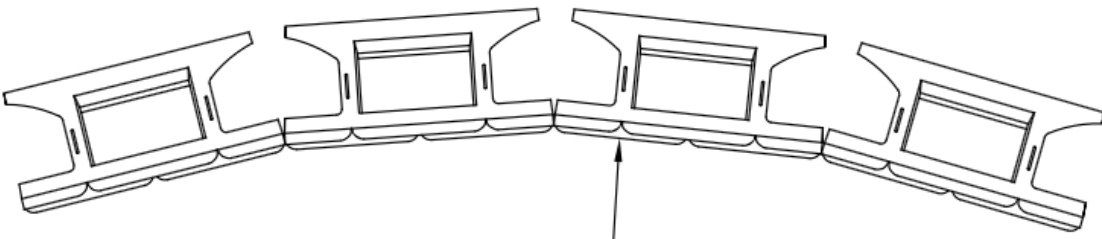
[TECH MEMOS](#)

| | |
|---|---------|
| GRAVITY ANALYSIS SPREADSHEET V5.6 (UPDATED 1.10.17) | PDF |
| 24 SF BLOCK TESTING REPORT | PDF |
| 6-28 SALES SHEET | PDF |
| 6 SF BLOCK TESTING REPORT | PDF |
| COMPONENTS METRIC | DWG PDF |
| COMPONENTS SAE | DWG PDF |
| CORNER DETAILS METRIC | DWG PDF |
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| CROSS SECTIONS METRIC | DWG PDF |
| CROSS SECTIONS SAE | DWG PDF |
| DETAILS INTRODUCTION | PDF |



Corners/Radius

24 SF Block



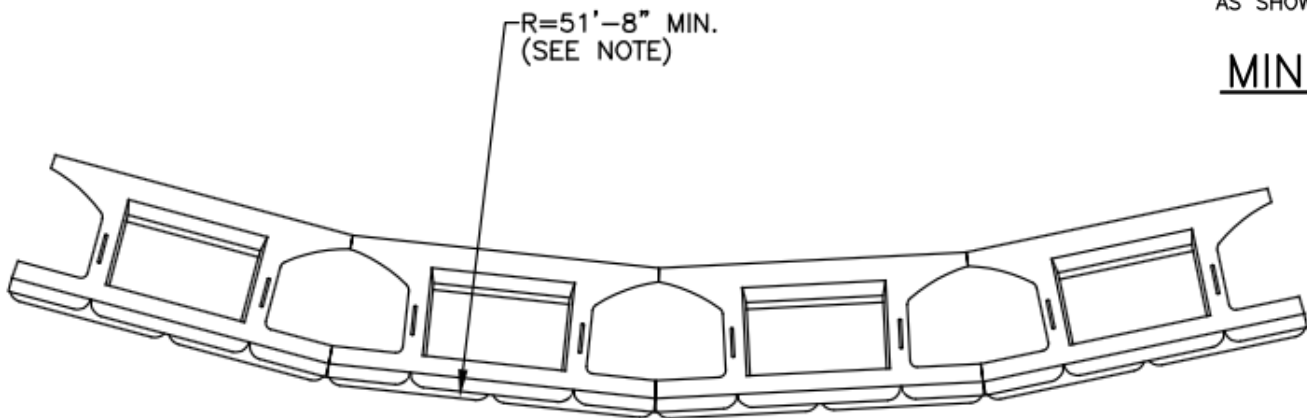
R=46' MIN.
(SEE NOTE)

NOTE:
MINIMUM RADIUS OCCURS AT LOWEST COURSE.
RADIUS INCREASES 4" PER COURSE ABOVE,
AS SHOWN ON TABLE.

| Minimum Concave Radius | | |
|------------------------|--------------------|----------------------------|
| Wall Height (ft) | Total # of Courses | Reqd. Radius at Top Course |
| 6 | 2 | 46' 4" |
| 9 | 3 | 46' 8" |
| 12 | 4 | 47' 0" |
| 15 | 5 | 47' 4" |
| 18 | 6 | 47' 8" |
| 21 | 7 | 48' 0" |
| 24 | 8 | 48' 4" |

MINIMUM CONCAVE RADIUS—24SF UNITS

NOT TO SCALE



R=51'-8" MIN.
(SEE NOTE)

NOTE:
MINIMUM RADIUS OCCURS AT TOP COURSE.
REQUIRED RADIUS INCREASES 4" PER COURSE
BELOW, AS SHOWN ON TABLE.

| Minimum Convex Radius | | |
|-----------------------|--------------------|------------------------------|
| Wall Height (ft) | Total # of Courses | Reqd. Radius at First Course |
| 6 | 2 | 52' 0" |
| 9 | 3 | 52' 4" |
| 12 | 4 | 52' 8" |
| 15 | 5 | 53' 0" |
| 18 | 6 | 53' 4" |
| 21 | 7 | 53' 8" |
| 24 | 8 | 54' 0" |

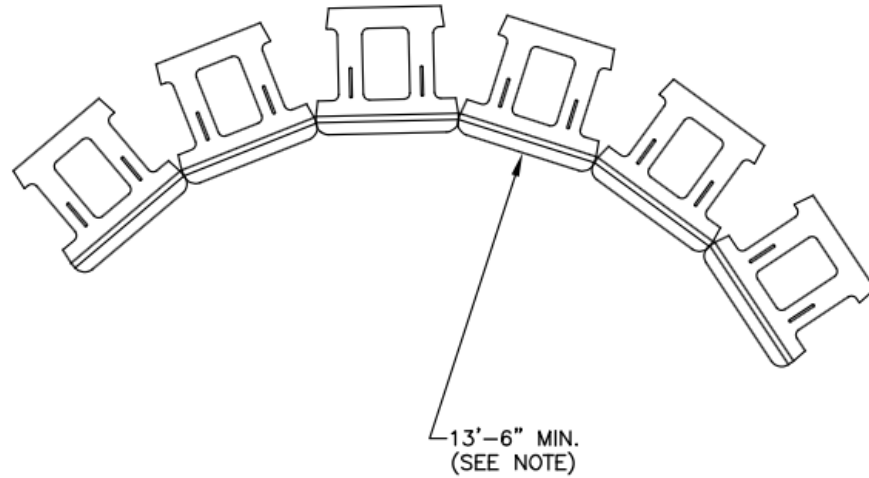
MINIMUM CONVEX RADIUS—24SF UNITS

NOT TO SCALE



Corners/Radius

6 SF Block



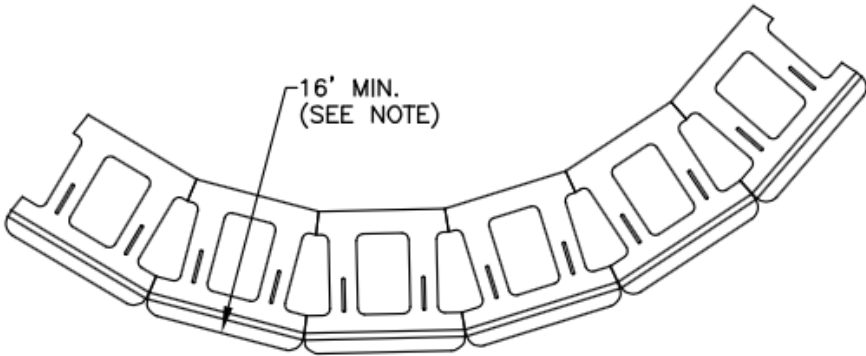
| Minimum Concave Radius | | |
|------------------------|--------------------|----------------------------|
| Wall Height (ft) | Total # of Courses | Reqd. Radius at Top Course |
| 3 | 2 | 13' 8" |
| 4 1/2 | 3 | 13' 10" |
| 6 | 4 | 14' 0" |
| 7 1/2 | 5 | 14' 2" |
| 9 | 6 | 14' 4" |
| 10 1/2 | 7 | 14' 6" |
| 12 | 8 | 14' 8" |

NOTE:
 MINIMUM RADIUS OCCURS AT LOWEST COURSE.
 RADIUS INCREASES 2" PER COURSE
 ABOVE, AS SHOWN ON TABLE.

MINIMUM CONCAVE RADIUS—6SF UNITS

NOT TO SCALE

| Minimum Convex Radius | | |
|-----------------------|--------------------|------------------------------|
| Wall Height (ft) | Total # of Courses | Reqd. Radius at First Course |
| 3 | 2 | 16' 2" |
| 4 1/2 | 3 | 16' 4" |
| 6 | 4 | 16' 6" |
| 7 1/2 | 5 | 16' 8" |
| 9 | 6 | 16' 10" |
| 10 1/2 | 7 | 17' 0" |
| 12 | 8 | 17' 2" |



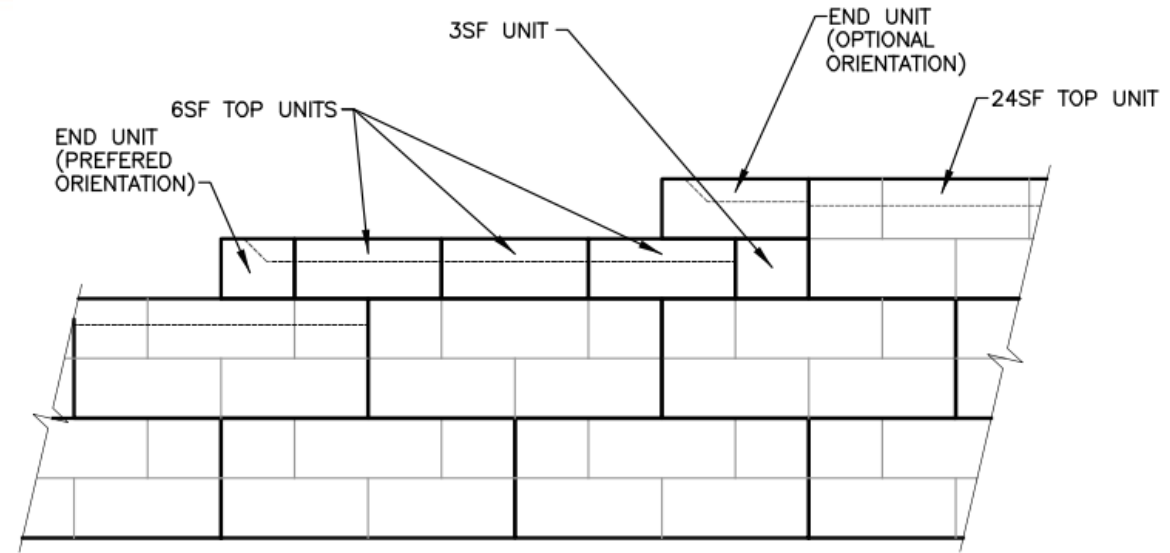
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 NOTE: MINIMUM RADIUS OCCURS AT TOP COURSE.
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 BELOW, AS SHOWN ON TABLE.

MINIMUM CONVEX RADIUS—6SF UNITS

NOT TO SCALE

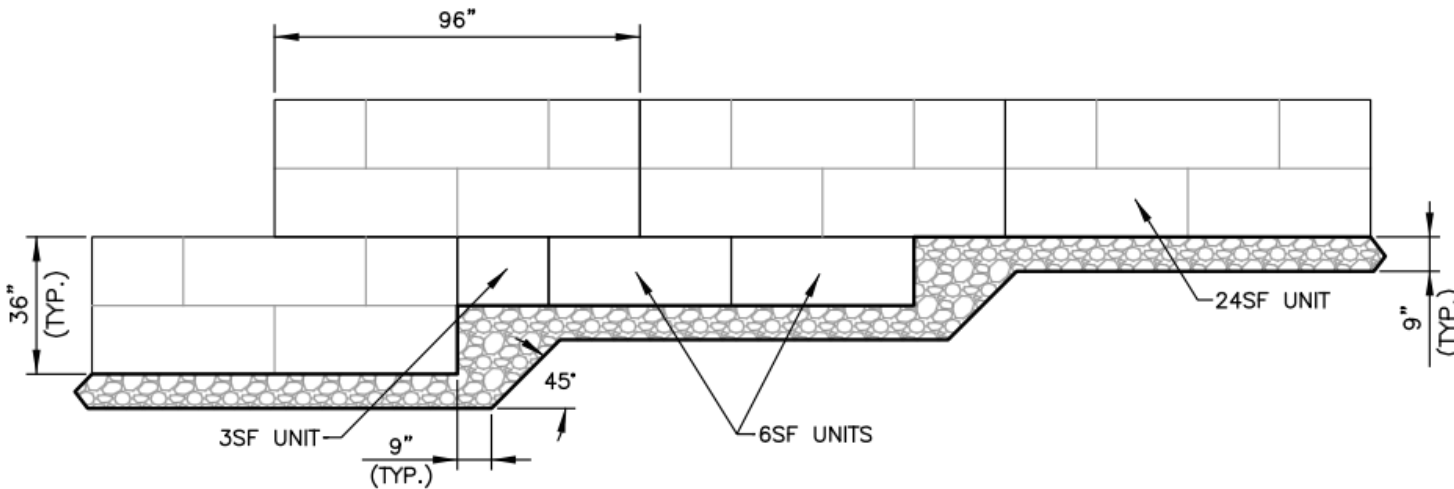


Top End/Steps



TOP OF WALL STEPS

NOT TO SCALE

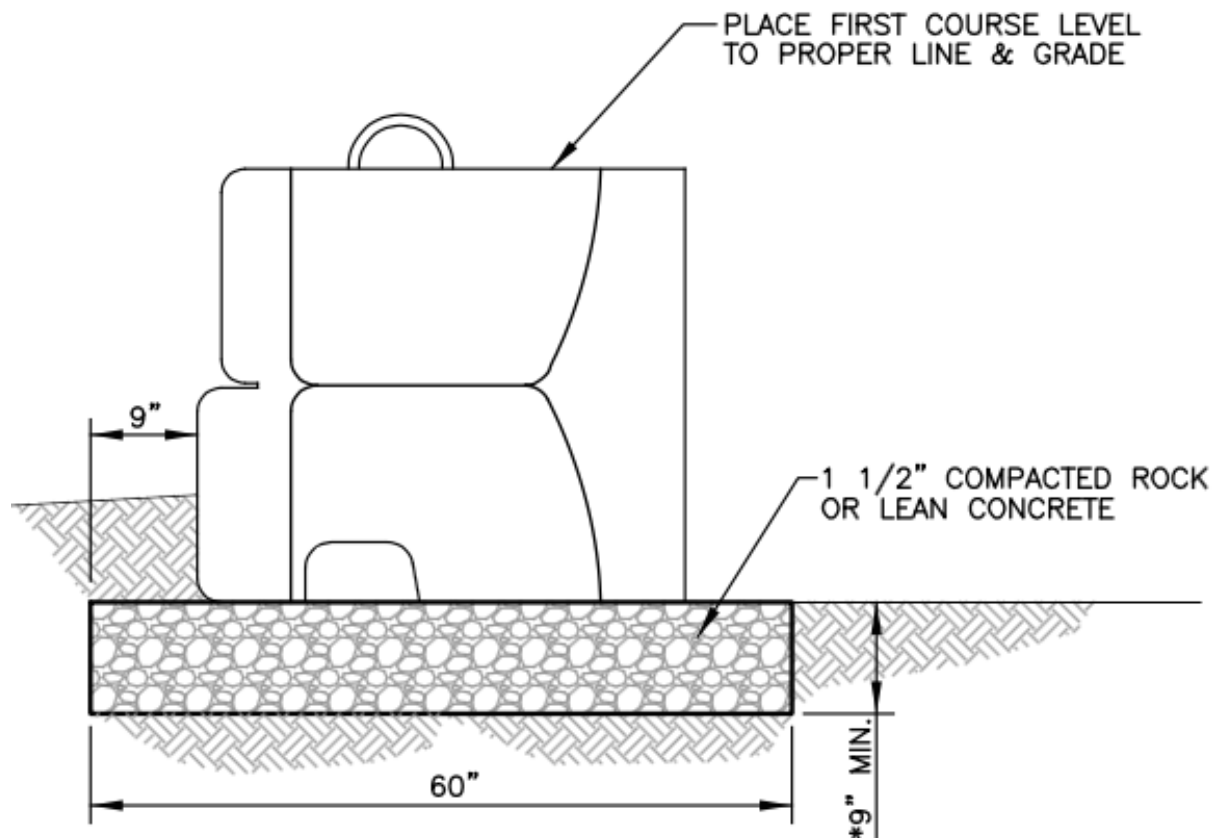


24SF WALL BASE STEP

NOT TO SCALE



Base/Embedment & Backfill Compaction



Proper backfill compaction is critical to wall stability

No reduced compaction requirement behind face

Less sensitive to displacement – 10,000 lb. mass can support equipment



THE THEORY



Coulomb's Active Earth Pressure Theory

$$K_a = \frac{\cos^2(\phi + \omega)}{\cos^2(\omega)\cos(\omega - \delta) \left[1 + \sqrt{\frac{\sin(\phi + \delta)\sin(\phi - \beta)}{\cos(\omega - \delta)\cos(\omega + \beta)}} \right]^2}$$



Reference Standards

- **AASHTO Bridge Manual**
- **FHWA-NHI-10-024, MSE Walls and Reinforced Soil Slopes**
- **CAN-CSA-S6-06, Canadian Highway Bridge Design Code**
- **NZTA SP/M/022, New Zealand Transport Agency Bridge Manual**
- **International Building Code**





Customer: "So, how high can we build"

Engineer: "Well, it depends..."



International Building Code

24SF & 6SF units only

| | |
|--|---|
| Soil Type | Backfill Slope |
| | Surcharge |
| | Seismic PGA |
| | Clay, $\phi = 26^\circ$ |
| Sand, $\phi = 30^\circ$ | |
| Sand/Gravel, $\phi = 34^\circ$ | |
| Crushed Stone, $\phi = 38^\circ$ | |

| Level | Level | Level | 3H:1V |
|----------|----------|----------|----------|
| 0 psf | 150 psf | 0 psf | 0 psf |
| 0g | 0g | 0.20g | 0g |
| 9.0 ft. | 7.5 ft. | 9.0 ft. | 7.5 ft. |
| 10.5 ft. | 7.5 ft. | 9.0 ft. | 7.5 ft. |
| 12.0 ft. | 10.5 ft. | 10.5 ft. | 10.5 ft. |
| 13.5 ft. | 10.5 ft. | 12.0 ft. | 12.0 ft. |

Table based on minimum recommended safety factors:
 Overturning FS=1.5 Sliding FS=1.5 Bearing FS=2.0
 Seismic safety factors reduced by 25%
 clay soil includes 150 psf cohesion in foundation soil
 unit weight 120 pcf for clay, 125 pcf for all other soils
 foundation soil limited to $\phi = 30^\circ$

24-62 base unit

| | |
|--|---|
| Soil Type | Backfill Slope |
| | Surcharge |
| | Seismic PGA |
| | Clay, $\phi = 26^\circ$ |
| Sand, $\phi = 30^\circ$ | |
| Sand/Gravel, $\phi = 34^\circ$ | |
| Crushed Stone, $\phi = 38^\circ$ | |

| Level | Level | Level | 3H:1V |
|----------|----------|----------|----------|
| 0 psf | 150 psf | 0 psf | 0 psf |
| 0g | 0g | 0.20g | 0g |
| 12.0 ft. | 10.5 ft. | 12.0 ft. | 9.0 ft. |
| 13.5 ft. | 10.5 ft. | 13.5 ft. | 10.5 ft. |
| 15.0 ft. | 13.5 ft. | 13.5 ft. | 13.5 ft. |
| 18.0 ft. | 16.5 ft. | 15.0 ft. | 16.5 ft. |

24-ME (12" extension) base unit

| Level | Level | Level | 3H:1V |
|----------|----------|----------|----------|
| 0 psf | 150 psf | 0 psf | 0 psf |
| 0g | 0g | 0.20g | 0g |
| 12.0 ft. | 9.0 ft. | 12.0 ft. | 9.0 ft. |
| 13.5 ft. | 10.5 ft. | 12.0 ft. | 10.5 ft. |
| 15.0 ft. | 13.5 ft. | 13.5 ft. | 13.5 ft. |
| 16.5 ft. | 15.0 ft. | 15.0 ft. | 15.0 ft. |

24-86 & 24-62 base units

| | |
|--|---|
| Soil Type | Backfill Slope |
| | Surcharge |
| | Seismic PGA |
| | Clay, $\phi = 26^\circ$ |
| Sand, $\phi = 30^\circ$ | |
| Sand/Gravel, $\phi = 34^\circ$ | |
| Crushed Stone, $\phi = 38^\circ$ | |

| Level | Level | Level | 3H:1V |
|----------|----------|----------|----------|
| 0 psf | 150 psf | 0 psf | 0 psf |
| 0g | 0g | 0.20g | 0g |
| 15.0 ft. | 12.0 ft. | 15.0 ft. | 10.5 ft. |
| 16.5 ft. | 13.5 ft. | 16.5 ft. | 12.0 ft. |
| 19.5 ft. | 16.5 ft. | 18.0 ft. | 15.0 ft. |
| 22.5 ft. | 19.5 ft. | 19.5 ft. | 19.5 ft. |

24" CIP tail extension

| Level | Level | Level | 3H:1V |
|----------|----------|----------|----------|
| 0 psf | 150 psf | 0 psf | 0 psf |
| 0g | 0g | 0.20g | 0g |
| 13.5 ft. | 10.5 ft. | 13.5 ft. | 10.5 ft. |
| 15.0 ft. | 13.5 ft. | 15.0 ft. | 12.0 ft. |
| 18.0 ft. | 16.5 ft. | 15.0 ft. | 15.0 ft. |
| 19.5 ft. | 18.0 ft. | 16.5 ft. | 18.0 ft. |



AASHTO LRFD

24SF & 6SF units only

Soil Type

Backfill Slope
Surcharge
Seismic PGA

| Level | Level | Level | 3H:1V |
|----------|---------|----------|----------|
| 0 psf | 250 psf | 0 psf | 0 psf |
| 0g | 0g | 0.20g | 0g |
| 7.5 ft. | 4.5 ft. | 7.5 ft. | 6.0 ft. |
| 9.0 ft. | 4.5 ft. | 7.5 ft. | 7.5 ft. |
| 10.5 ft. | 7.5 ft. | 10.5 ft. | 9.0 ft. |
| 12.0 ft. | 9.0 ft. | 12.0 ft. | 10.5 ft. |

Clay, $\phi = 26^\circ$

Sand, $\phi = 30^\circ$

Sand/Gravel, $\phi = 34^\circ$

Crushed Stone, $\phi = 38^\circ$

24-86 & 24-62 base units

Soil Type

Backfill Slope
Surcharge
Seismic PGA

| Level | Level | Level | 3H:1V |
|----------|----------|----------|----------|
| 0 psf | 250 psf | 0 psf | 0 psf |
| 0g | 0g | 0.20g | 0g |
| 13.5 ft. | 10.5 ft. | 13.5 ft. | 10.5 ft. |
| 16.5 ft. | 12.0 ft. | 16.5 ft. | 12.0 ft. |
| 18.0 ft. | 15.0 ft. | 18.0 ft. | 15.0 ft. |
| 19.5 ft. | 16.5 ft. | 19.5 ft. | 18.0 ft. |

Clay, $\phi = 26^\circ$

Sand, $\phi = 30^\circ$

Sand/Gravel, $\phi = 34^\circ$

Crushed Stone, $\phi = 38^\circ$



AASHTO LRFD – Vertical Face

24SF & 6SF units only

| Soil Type | Backfill Slope | Level | Level | Level | 3H:1V |
|----------------------------------|----------------|----------|---------|----------|----------|
| | Surcharge | 0 psf | 250 psf | 0 psf | 0 psf |
| | Seismic PGA | 0g | 0g | 0.20g | 0g |
| | | 7.5 ft. | 4.5 ft. | 7.5 ft. | 7.5 ft. |
| Clay, $\phi = 26^\circ$ | | 7.5 ft. | 4.5 ft. | 7.5 ft. | 7.5 ft. |
| Sand, $\phi = 30^\circ$ | | 9.0 ft. | 7.5 ft. | 9.0 ft. | 9.0 ft. |
| Sand/Gravel, $\phi = 34^\circ$ | | 10.5 ft. | 7.5 ft. | 10.5 ft. | 10.5 ft. |
| Crushed Stone, $\phi = 38^\circ$ | | | | | |

24-86 & 24-62 base units

| Soil Type | Backfill Slope | Level | Level | Level | 3H:1V |
|----------------------------------|----------------|----------|----------|----------|----------|
| | Surcharge | 0 psf | 250 psf | 0 psf | 0 psf |
| | Seismic PGA | 0g | 0g | 0.20g | 0g |
| | | 13.5 ft. | 10.5 ft. | 13.5 ft. | 10.5 ft. |
| Clay, $\phi = 26^\circ$ | | 15.0 ft. | 12.0 ft. | 15.0 ft. | 12.0 ft. |
| Sand, $\phi = 30^\circ$ | | 15.0 ft. | 13.5 ft. | 15.0 ft. | 13.5 ft. |
| Sand/Gravel, $\phi = 34^\circ$ | | 16.5 ft. | 13.5 ft. | 16.5 ft. | 15.0 ft. |
| Crushed Stone, $\phi = 38^\circ$ | | | | | |

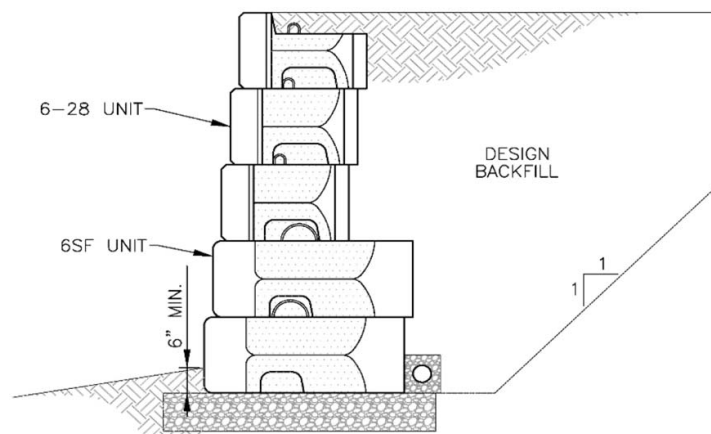


**Load Case 1 - Level Backfill
(Battered Face)**

Backslope: nearly level (or sloping away from wall)

Surcharge: 25 psf (nominal surcharge/snow load)

Based on IBC safety factors, 1.5 for sliding/overturning



Cohesive Backfill*

$\phi=26^\circ$, $c=100\text{psf}$, $\gamma=125\text{pcf}$

| | Total Wall Height (feet) | | | | | |
|---------------|--------------------------|------|------|------|-----|------|
| | 3.0 | 4.5 | 6.0 | 7.5 | 9.0 | 10.5 |
| 7th Course | | | | | | -- |
| 6th Course | | | | | -- | -- |
| 5th Course | | | | 6-28 | -- | -- |
| 4th Course | | | 6-28 | 6-28 | -- | -- |
| 3rd Course | | 6-28 | 6-28 | 6SF | -- | -- |
| 2nd Course | 6-28 | 6-28 | 6SF | 6SF | -- | -- |
| Bottom Course | 6-28 | 6-28 | 6SF | 6SF | -- | -- |

*design for soil within 1 foot of heel, extending up at 1H:1V slope, assumes clay foundation soil

Coarse Sand Backfill*

$\phi=32^\circ$, $c=0\text{psf}$, $\gamma=125\text{pcf}$

| | Total Wall Height (feet) | | | | | |
|---------------|--------------------------|------|------|------|------|------|
| | 3.0 | 4.5 | 6.0 | 7.5 | 9.0 | 10.5 |
| 7th Course | | | | | | 6-28 |
| 6th Course | | | | | 6-28 | 6-28 |
| 5th Course | | | | 6-28 | 6-28 | 6-28 |
| 4th Course | | | 6-28 | 6-28 | 6-28 | 6SF |
| 3rd Course | | 6-28 | 6-28 | 6-28 | 6SF | 6SF |
| 2nd Course | 6-28 | 6-28 | 6-28 | 6SF | 6SF | 6SF |
| Bottom Course | 6-28 | 6-28 | 6-28 | 6SF | 6SF | 6SF |

*design for soil within 1 foot of heel, extending up at 1H:1V slope, assumes sand foundation soil

Sand Backfill*

$\phi=30^\circ$, $c=0\text{psf}$, $\gamma=125\text{pcf}$

| | Total Wall Height (feet) | | | | | |
|---------------|--------------------------|------|------|------|------|------|
| | 3.0 | 4.5 | 6.0 | 7.5 | 9.0 | 10.5 |
| 7th Course | | | | | | -- |
| 6th Course | | | | | 6-28 | -- |
| 5th Course | | | | 6-28 | 6-28 | -- |
| 4th Course | | | 6-28 | 6-28 | 6-28 | -- |
| 3rd Course | | 6-28 | 6-28 | 6-28 | 6SF | -- |
| 2nd Course | 6-28 | 6-28 | 6-28 | 6SF | 6SF | -- |
| Bottom Course | 6-28 | 6-28 | 6-28 | 6SF | 6SF | -- |

*design for soil within 1 foot of heel, extending up at 1H:1V slope, assumes sand foundation soil

Gravel Backfill*

$\phi=34^\circ$, $c=0\text{psf}$, $\gamma=125\text{pcf}$

| | Total Wall Height (feet) | | | | | |
|---------------|--------------------------|------|------|------|------|------|
| | 3.0 | 4.5 | 6.0 | 7.5 | 9.0 | 10.5 |
| 7th Course | | | | | | 6-28 |
| 6th Course | | | | | 6-28 | 6-28 |
| 5th Course | | | | 6-28 | 6-28 | 6-28 |
| 4th Course | | | 6-28 | 6-28 | 6-28 | 6-28 |
| 3rd Course | | 6-28 | 6-28 | 6-28 | 6-28 | 6SF |
| 2nd Course | 6-28 | 6-28 | 6-28 | 6-28 | 6SF | 6SF |
| Bottom Course | 6-28 | 6-28 | 6-28 | 6SF | 6SF | 6SF |

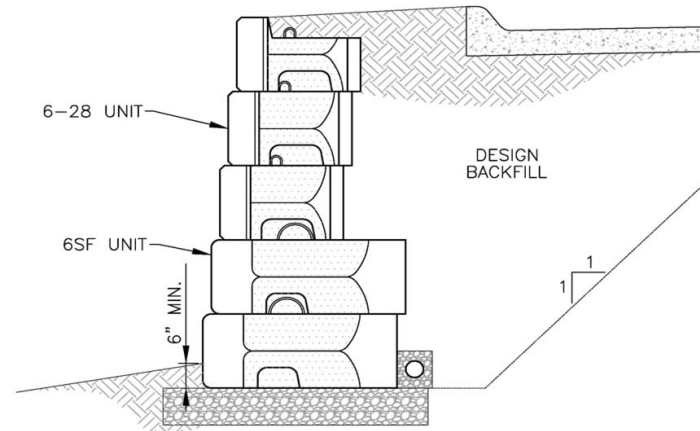
*design for soil within 1 foot of heel, extending up at 1H:1V slope, assumes sand foundation soil

Load Case 2 - Parking Lot Surcharge (Battered Face)

Backslope: nearly level (or sloping away from wall)

Surcharge: 150 psf (parking lot, set back min 2 feet behinds)

Based on IBC safety factors, 1.5 for sliding/overturning



Cohesive Backfill*

$\phi=26^\circ$, $c=100\text{psf}$, $\gamma=125\text{pcf}$

| | Total Wall Height (feet) | | | | | |
|---------------|--------------------------|------|------|-----|-----|------|
| | 3.0 | 4.5 | 6.0 | 7.5 | 9.0 | 10.5 |
| 7th Course | | | | | | -- |
| 6th Course | | | | | -- | -- |
| 5th Course | | | | -- | -- | -- |
| 4th Course | | | 6-28 | -- | -- | -- |
| 3rd Course | | 6-28 | 6-28 | -- | -- | -- |
| 2nd Course | 6-28 | 6-28 | 6SF | -- | -- | -- |
| Bottom Course | 6-28 | 6-28 | 6SF | -- | -- | -- |

*design for soil within 1 foot of heel, extending up at 1H:1V slope, assumes clay foundation soil

Coarse Sand Backfill*

$\phi=32^\circ$, $c=0\text{psf}$, $\gamma=125\text{pcf}$

| | Total Wall Height (feet) | | | | | |
|---------------|--------------------------|------|------|------|-----|------|
| | 3.0 | 4.5 | 6.0 | 7.5 | 9.0 | 10.5 |
| 7th Course | | | | | | -- |
| 6th Course | | | | | -- | -- |
| 5th Course | | | | 6-28 | -- | -- |
| 4th Course | | | 6-28 | 6-28 | -- | -- |
| 3rd Course | | 6-28 | 6-28 | 6-28 | -- | -- |
| 2nd Course | 6-28 | 6-28 | 6-28 | 6SF | -- | -- |
| Bottom Course | 6-28 | 6-28 | 6SF | 6SF | -- | -- |

*design for soil within 1 foot of heel, extending up at 1H:1V slope, assumes sand foundation soil

Sand Backfill*

$\phi=30^\circ$, $c=0\text{psf}$, $\gamma=125\text{pcf}$

| | Total Wall Height (feet) | | | | | |
|---------------|--------------------------|------|------|------|-----|------|
| | 3.0 | 4.5 | 6.0 | 7.5 | 9.0 | 10.5 |
| 7th Course | | | | | | -- |
| 6th Course | | | | | -- | -- |
| 5th Course | | | | 6-28 | -- | -- |
| 4th Course | | | 6-28 | 6-28 | -- | -- |
| 3rd Course | | 6-28 | 6-28 | 6-28 | -- | -- |
| 2nd Course | 6-28 | 6-28 | 6-28 | 6SF | -- | -- |
| Bottom Course | 6-28 | 6-28 | 6SF | 6SF | -- | -- |

*design for soil within 1 foot of heel, extending up at 1H:1V slope, assumes sand foundation soil

Gravel Backfill*

$\phi=34^\circ$, $c=0\text{psf}$, $\gamma=125\text{pcf}$

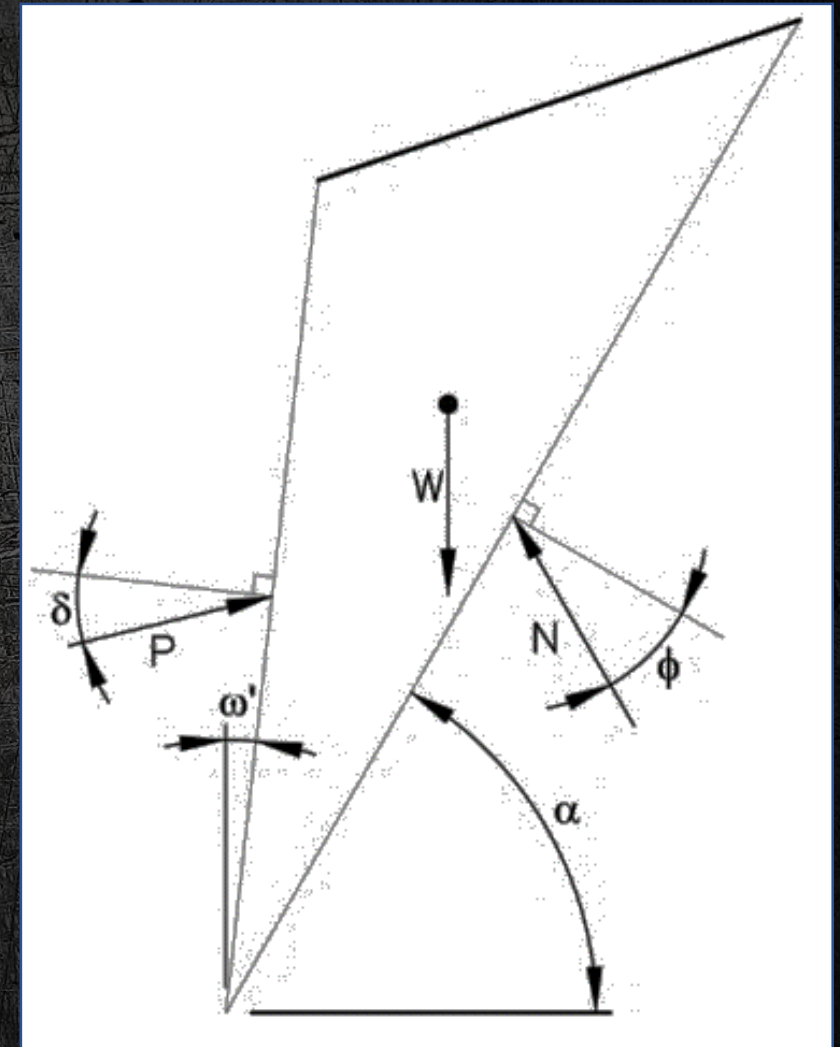
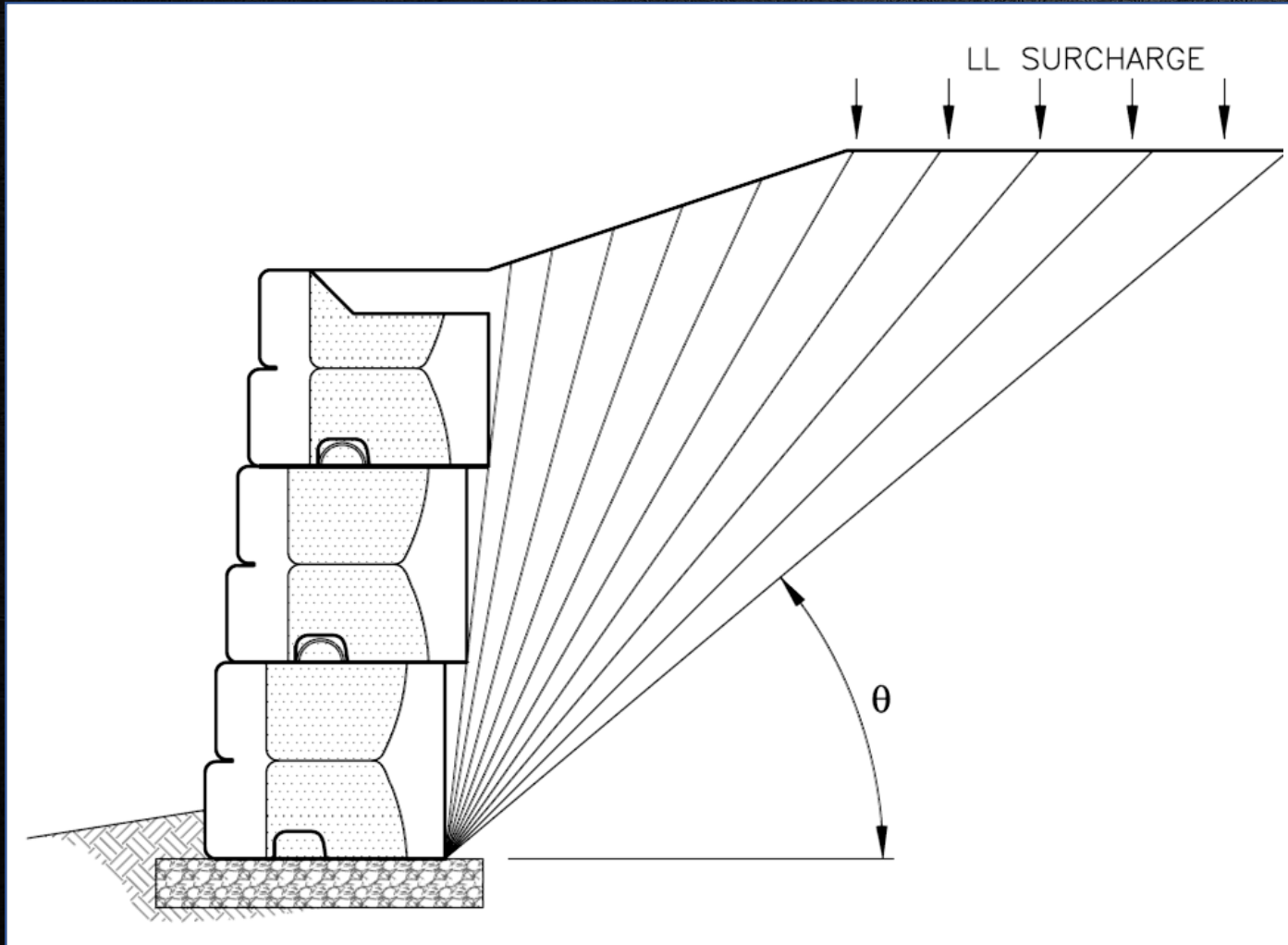
| | Total Wall Height (feet) | | | | | |
|---------------|--------------------------|------|------|------|------|------|
| | 3.0 | 4.5 | 6.0 | 7.5 | 9.0 | 10.5 |
| 7th Course | | | | | | -- |
| 6th Course | | | | | 6-28 | -- |
| 5th Course | | | | 6-28 | 6-28 | -- |
| 4th Course | | | 6-28 | 6-28 | 6-28 | -- |
| 3rd Course | | 6-28 | 6-28 | 6-28 | 6SF | -- |
| 2nd Course | 6-28 | 6-28 | 6-28 | 6SF | 6SF | -- |
| Bottom Course | 6-28 | 6-28 | 6-28 | 6SF | 6SF | -- |

*design for soil within 1 foot of heel, extending up at 1H:1V slope, assumes sand foundation soil



Trial Wedge Method

Variation of Coulomb Technique



Not commonly used due to computational effort.

“Fine, we’ll create a solution ourselves.”



Defining Complex Boundary Conditions

Seismic Load Ss **0.20** G site class (A to E or 1) **D** Fa 1.60 k_n 0.03

Backfill Slope & Surcharge

| | | | | | | | |
|------------------------|-----------------------------|-------------------|-------------|--------------|----------------|-------------|----|
| length 1 | 11 feet (horizontal) | rise in grade | 2 ft | LL surcharge | psf | tier height | ft |
| length 2 | 14 feet (horizontal) | | ft | | 150 psf | | ft |
| length 3 | ft (horizontal) | | ft | | psf | | ft |
| length 4 | ft (horizontal) | | ft | | psf | | ft |
| effective slope | 4.11 H:1V slope | β | 13.7 deg | avg q | | 52 psf | |
| failure plane α | 53.58 deg | zone of influence | 14.39 ft | | | | |

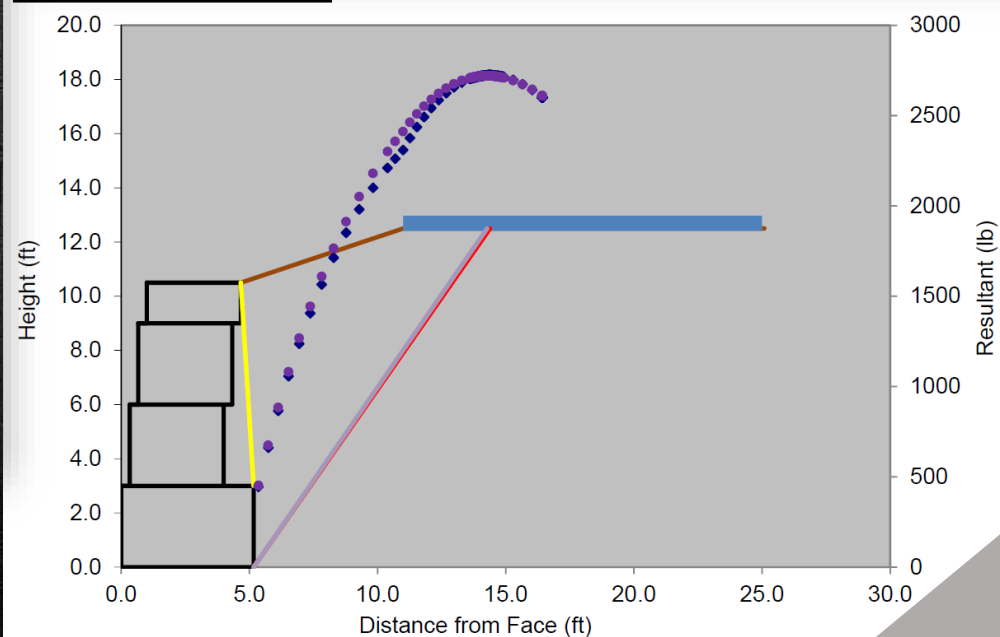
Analysis

| | | | | | | | |
|-------|-----------|-----------|-------------|------------------|----------|-----------|-----------|
| K_a | = 0.368 | Q_{in} | = 168 lb | ΔK_{AE} | = 0.027 | e | = 0.80 ft |
| P_h | = 2265 lb | Q_{iv} | = 84 lb | P_{IR} | = 167 lb | B'_i | = 4.14 ft |
| P_v | = 1140 lb | R_s | = 4019 lb | ΔP_{AEh} | = 165 lb | e_{eq} | = 0.89 ft |
| | | q_{ult} | = 9,251 psf | ΔP_{AEv} | = 83 lb | B'_{eq} | = 3.96 ft |

Results

| | | |
|-----------------------------|-----------------------|-----------------------------|
| Overturning: | Desired FS = 1.5 | Actual FS = 2.16 OK! |
| Sliding: | Desired FS = 1.5 | Actual FS = 1.65 OK! |
| Bearing Capacity: | Desired FS = 2 | Actual FS = 5.16 OK! |
| | $q_{all} = 4,626$ psf | $q_c = 1,794$ psf |
| Seismic Overturning: | Desired FS = 1.13 | Actual FS = 2.03 OK! |
| Seismic Sliding: | Desired FS = 1.13 | Actual FS = 1.61 OK! |
| Seismic Bearing: | Desired FS = 1.5 | Actual FS = 4.86 OK! |
| | $q_{all} = 5,999$ psf | $q_c = 1,871$ psf |

Ground Surface & Trial Wedge Plot



STONE STRONG GRAVITY CALCULATIONS - ver 5.3

Project Name: Gravity Retaining Wall
 Location: Omaha, NE
 Job#:
 Section: Example
 Calc by: D Thiele

Page 1 of 3
8/9/16 10:47

Notes

| Wall Configuration | | setback (in) | | modular units | | unit fill | | soil wedge | | CIP Extension | | Internal Stability FS | | Seismic Internal FS | | | |
|--------------------|--------|--------------|------|---------------|---------|-----------|---------|------------|---------|---------------|---------|-----------------------|--------|---------------------|--------|-------|-----|
| unit | w (in) | h (ft) | face | tail | Wb (lb) | xb (in) | Wa (lb) | xa (in) | Ws (lb) | xs (in) | we (in) | h _i | Topple | Shear | Topple | Shear | |
| 6 | 44.0 | 1.50 | 12.0 | -6.0 | 400 | 31.0 | 296 | 33.5 | 8 | 54.4 | | | 43.71 | 17.93 | 25.84 | 12.33 | OK! |
| 24 | 44.0 | 3.00 | 8.0 | -10.0 | 750 | 27.2 | 594 | 30.8 | 176 | 53.2 | | | 6.16 | 4.95 | 4.69 | 4.13 | OK! |
| 24 | 44.0 | 3.00 | 4.0 | -14.0 | 750 | 23.2 | 594 | 26.8 | 352 | 52.4 | | | 2.40 | 2.70 | 2.07 | 2.46 | OK! |
| 24-62 | 62.0 | 3.00 | 0.0 | 0.0 | 857 | 27.1 | 1,043 | 31.0 | 0 | 0.0 | | | | | | | |

External Stability OK!

| | | | | | |
|-----------------|-------------------|-------------|-----------|--------------------------|----------|
| backfill height | 10.50 feet | $\omega =$ | 6.34 deg | interface friction angle | |
| exposed height | 9.75 feet | $\omega' =$ | -2.73 deg | δ | 24.0 deg |

Retained Soil

γ **125** pcf
 ϕ **32** deg

Foundation Soil

γ **125** pcf
 c' **30** psf
 ϕ **30** deg

(if specified)
allowable bearing pressure **n/a** psf (net)

base embedment **9** in
base thickness **9** in
base material **agg**
toe slope **1** H:1V slope

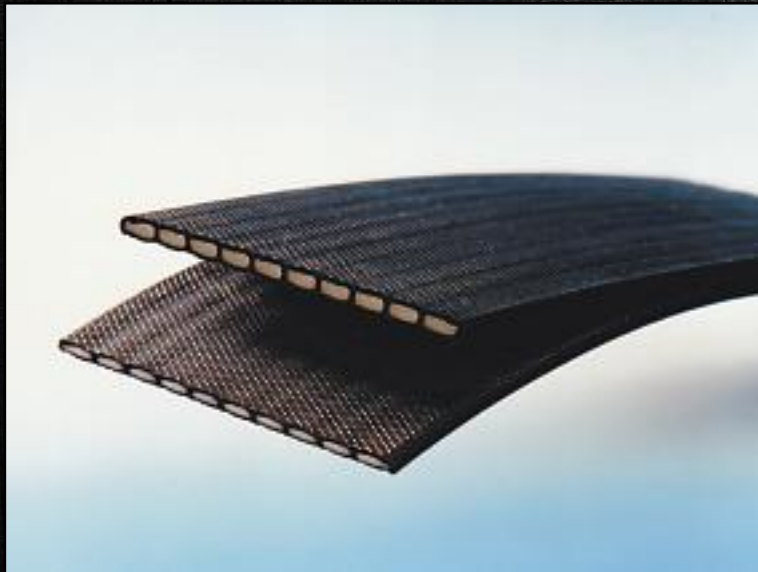
composite friction coefficient μ_b 0.69

© S T O N E S T R O N G S Y S T E M S



Stone Strong / Paraweb

- **Conforms to AASHTO standards**
- **Innovative MSE system**
- **Polyester yarn bundles encased in tough and durable polyethylene sheath**
- **Positive connection**
- **Stable face**



Interaction Testing Reports

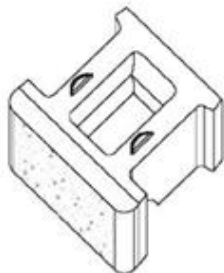
24SF Block & 6SF Block



Interaction Testing Report

6 SF Units with Synten Geogrids

**Stone Strong Systems
Lincoln, Nebraska**



Prepared for:
Stone Strong Systems
3801 Union Drive, Suite 102
Lincoln, Nebraska 68516

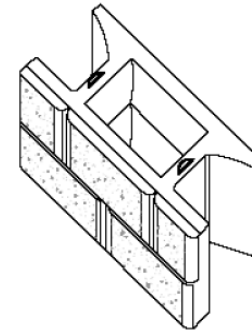
May 27, 2005
TG Project No. 02546.2



Interaction Testing Report

24 SF Units with Synten Geogrids

**Stone Strong Systems
Lincoln, Nebraska**



Prepared for:
Stone Strong Systems
3801 Union Drive, Suite 102
Lincoln, Nebraska 68516

September 17, 2004
TG Project No. 02546.2

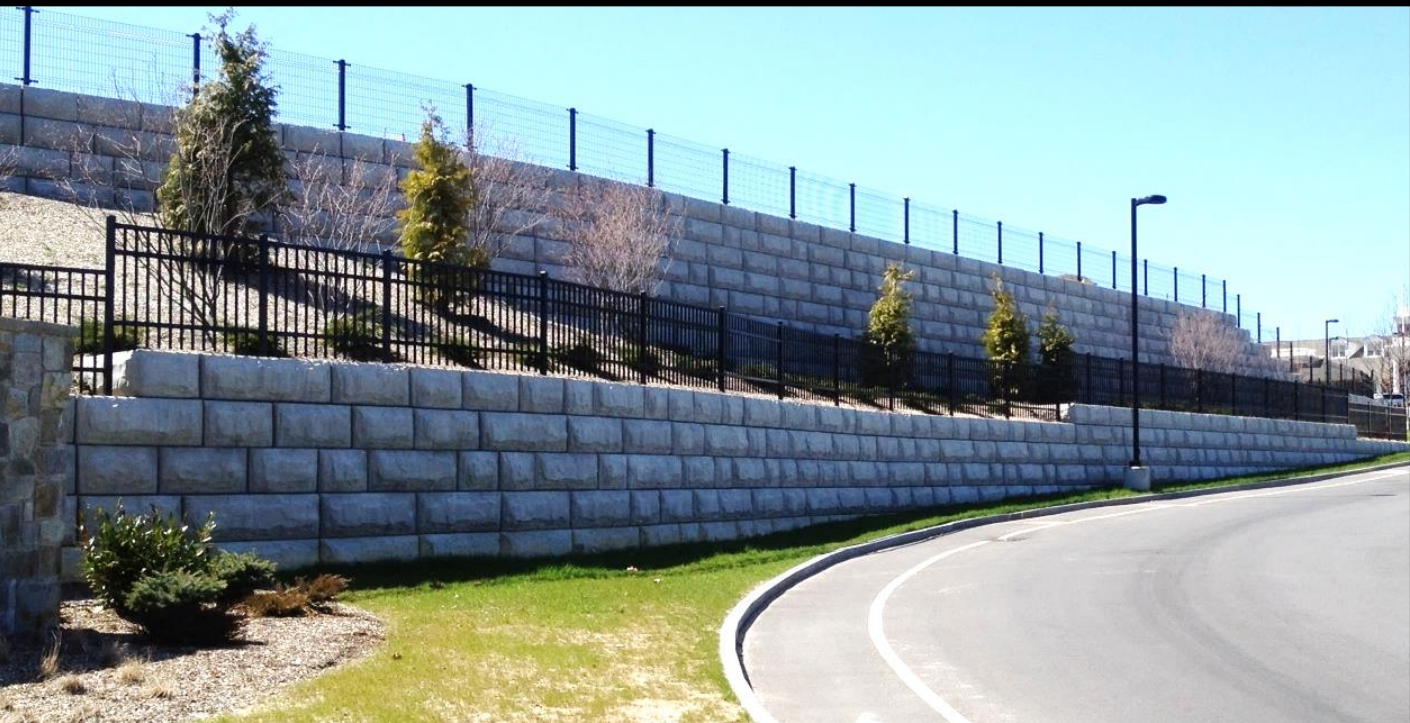


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systems
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PATENT NUMBERS 6,796,098 AND 7,073,504
BY: **ERMC**
Erie River Machine Company

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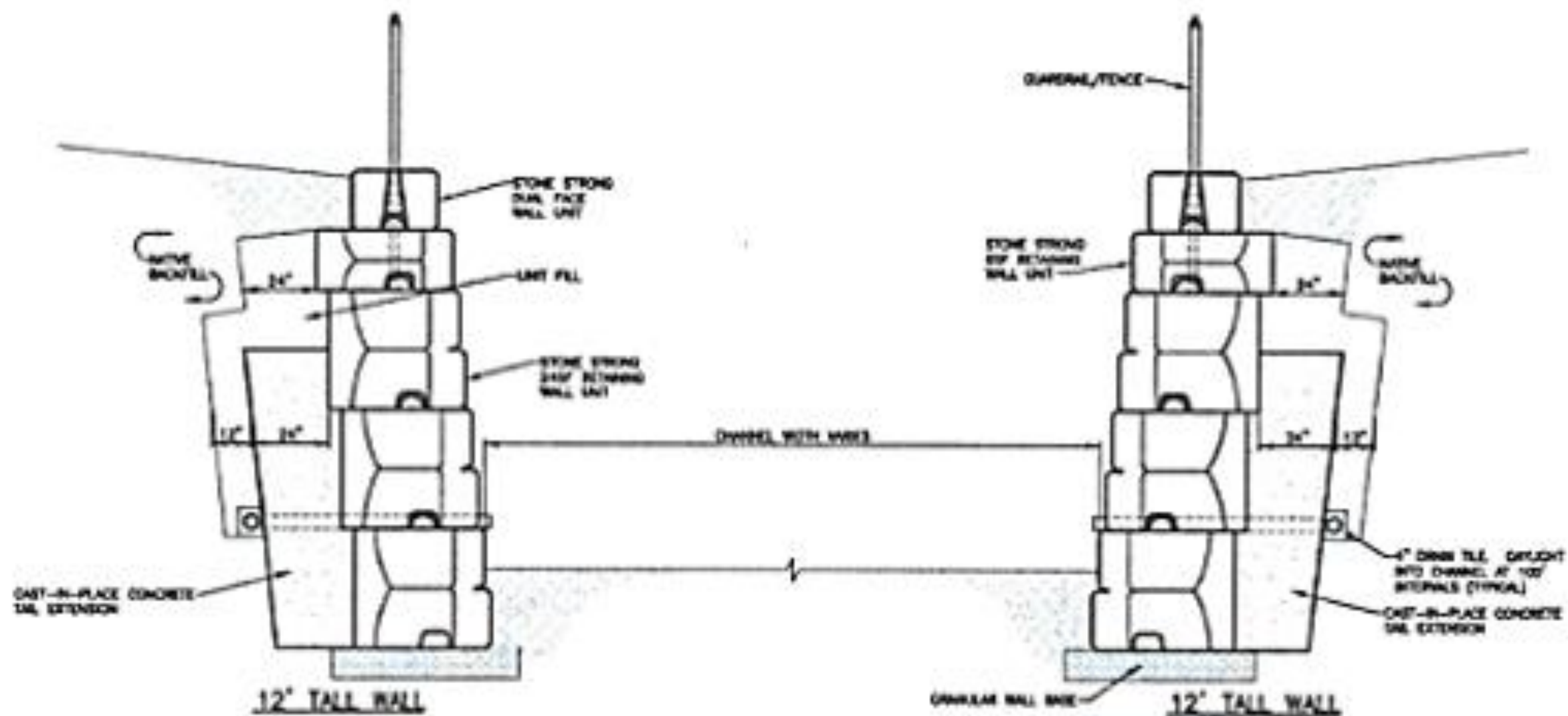


SHORELINE









CHANNEL CROSS SECTION
 NOT TO SCALE



Safety and Risk Mgmt.

Purchasing Division
432.685.7234
Fax 685.0523

Warehouse
432.685.7244
Fax 432.686.1648

Department of Facilities and Fleet Management
Post Office Box 1152
Midland, Texas 79702-1152

Fleet Services
432.685.7455

Service Center
432.685.7460

Facilities Services
432.685.7271

March 1, 2010

Stone Strong Systems
Attn: Jody DuBois
P. O. Box 835
Justin, TX 76247

Subject: Annual Purchase Agreement, PA0310-10, for Precast Concrete Retaining
Wall Blocks

Dear Mr. DuBois:

Attached you will find two copies of the purchase agreement transmitted previously with
the subject Request for Sealed Bid.

Please execute and **notarize** both purchase agreement copies and return them to this
office as soon as possible.

Upon receipt of all information in an acceptable manner, we shall execute the duplicate
copies of the agreement and return one copy for your files.

Please contact this office if you have any questions.

Sincerely,

A handwritten signature in black ink, appearing to read "Eddie Price", with a long horizontal flourish extending to the right.

Eddie Price, C.P.M.
Purchasing Agent



MATERIALS MANAGEMENT DIVISION • 901-B TEXAS STREET • DENTON, TEXAS 76209
940.349.7100 • FAX 940.349.7302

August 19, 2014

Jody DuBois
Stone Strong of Texas
PO Box 835
Justin, TX 76247
jody@stonestrongoftexas.com

Ref: File 4834- Renewal of Contract for Retaining Wall Blocks

Dear Jody:

The City of Denton would like to renew its contract with Stone Strong of Texas for the above referenced File for an additional one-year period through October 18, 2015, if agreed to by both parties, with all prices, terms and conditions remaining the same. If your company is in agreement, please sign a copy of this letter and return it to me.

Thank you for your continued interest in the City of Denton. If you have any questions, please contact me.

Regards,

Elton D. Brock, MBA, CTPM, CTCM, C.P.M.
Purchasing Manager

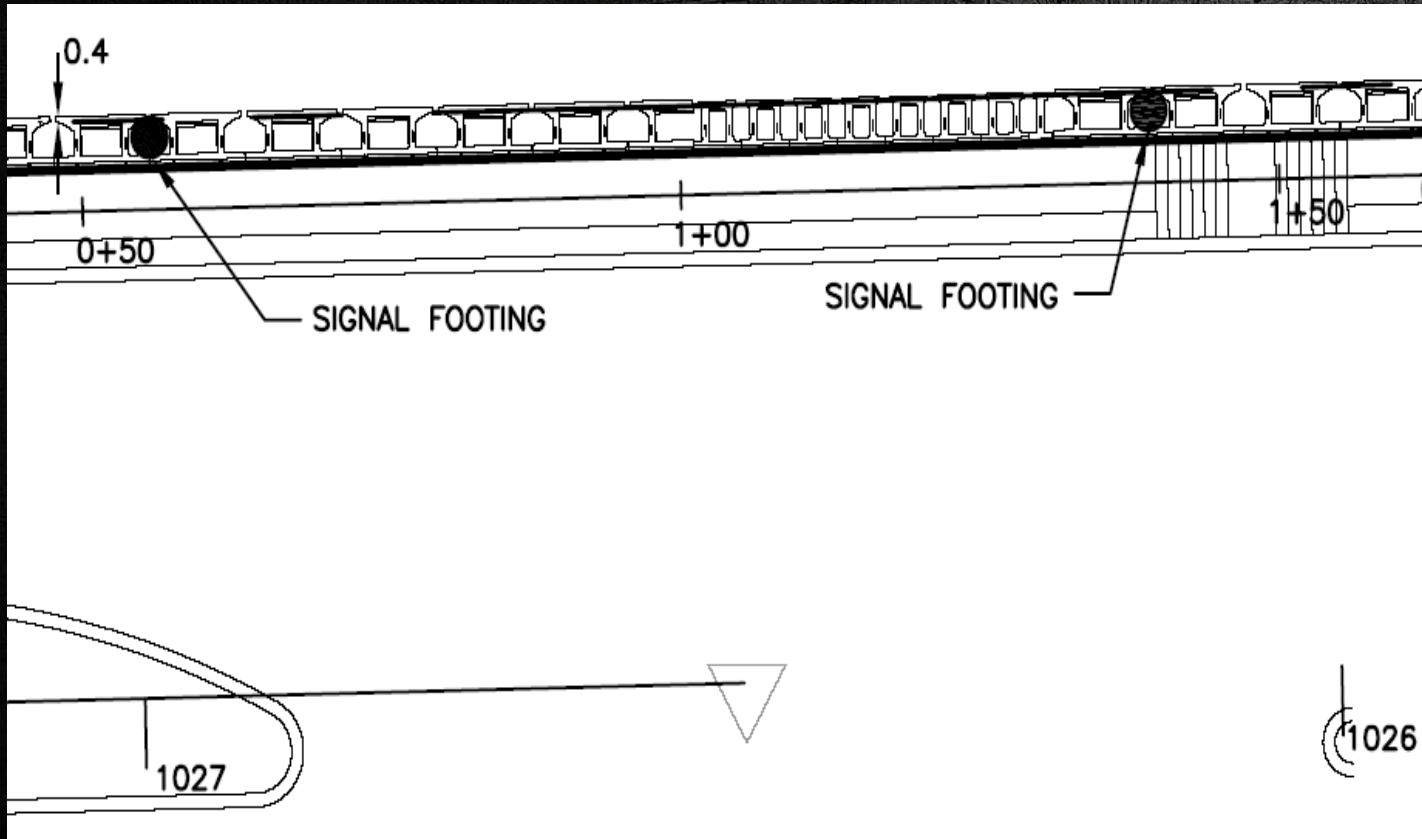
Authorized Rep.

8/20/2014

Date

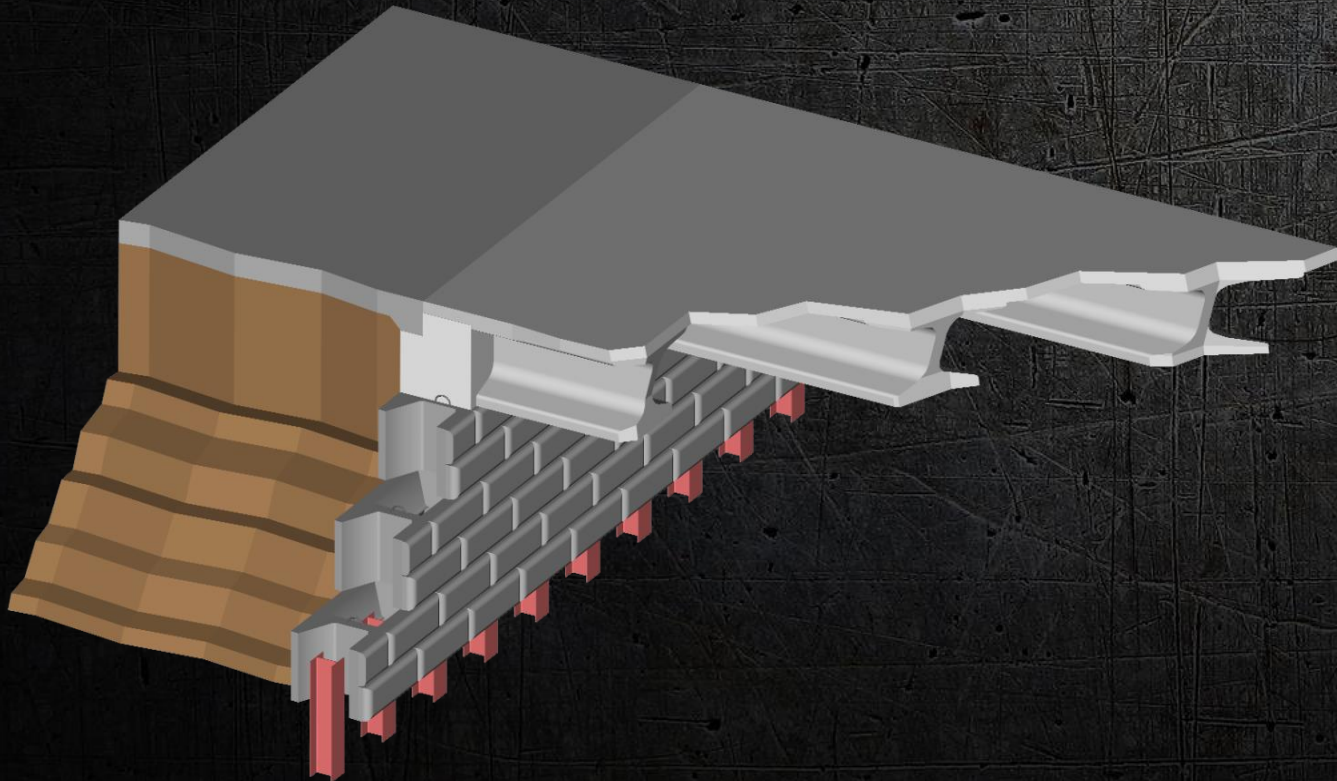
Integrated Structures

Harrison Street | Omaha, Nebraska

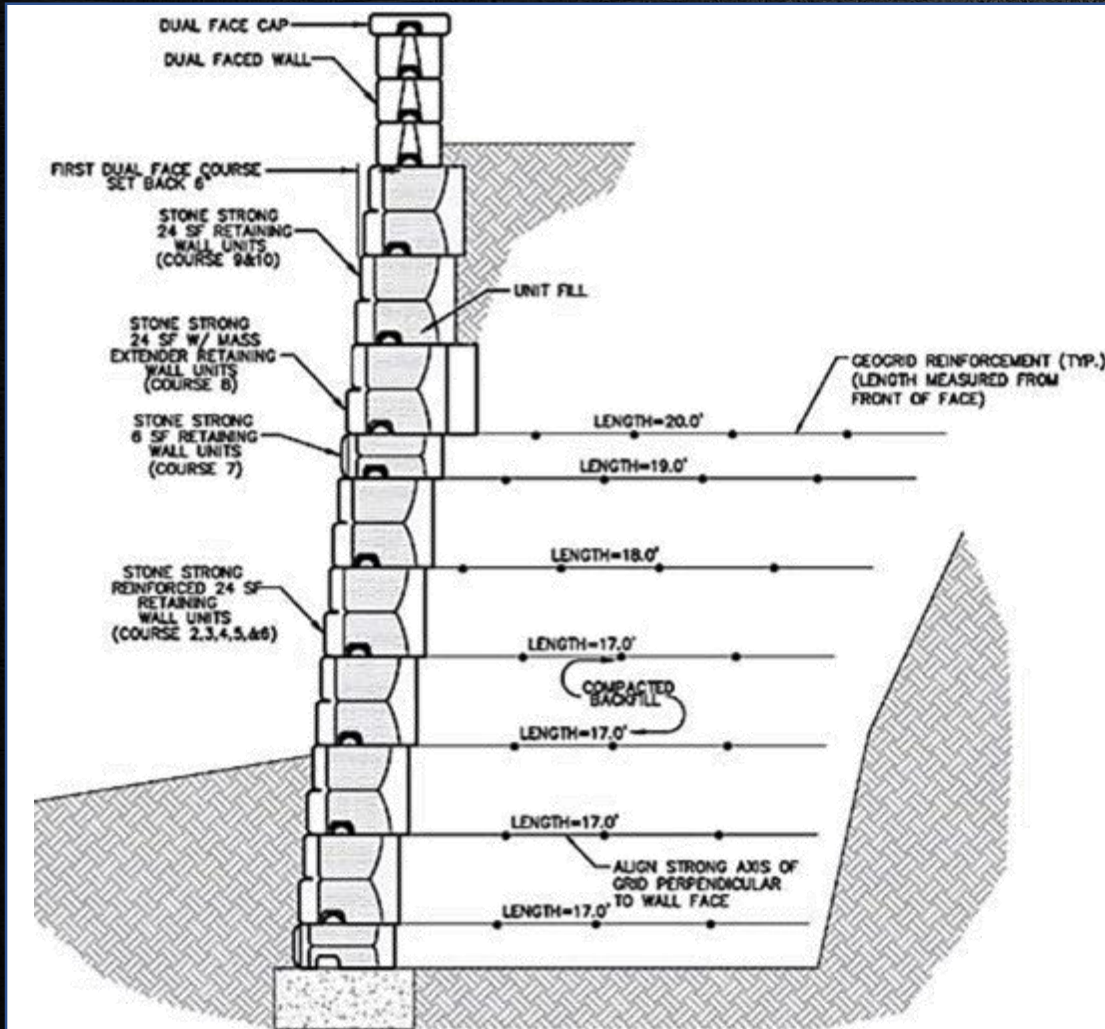


Bridge Abutments

Ulster County | Kingston, New York



Hybrid Applications



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